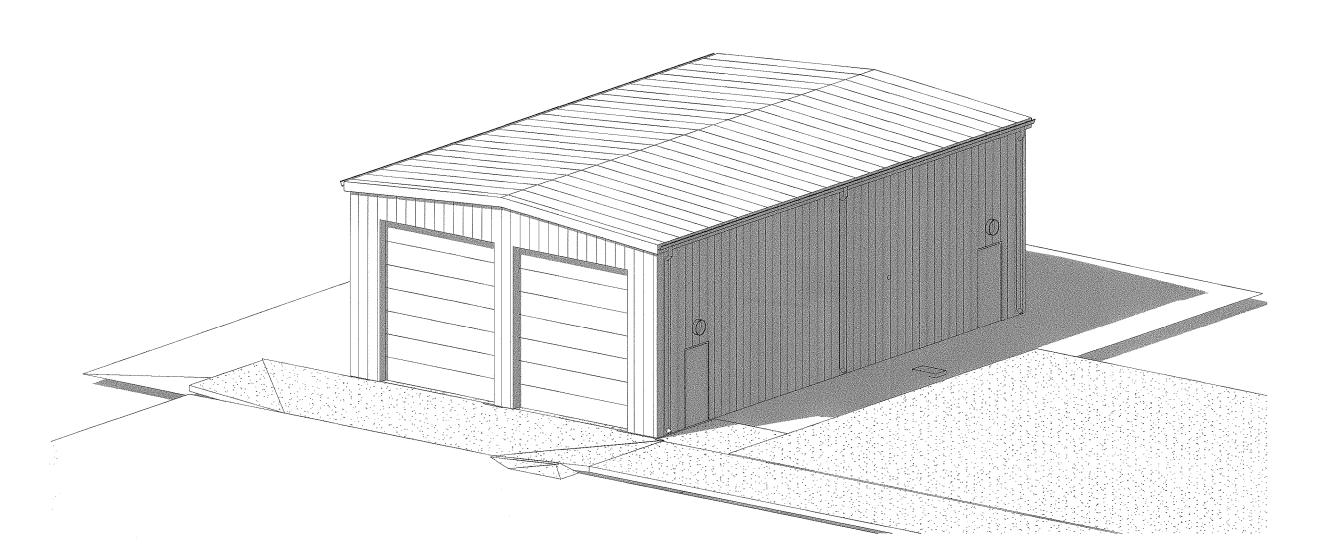
NOTE: THESE CONSTRUCTION DOCUMENTS ARE ISSUED TO SHOW DESIGN INTENT AND ESTABLISH A MATERIAL STANDARD FOR PRICING. THE CONTRACTOR IS RESPONSIBLE TO PROVIDE AN INDUSTRY STANDARD OF CARE AND EXERCISE COMPETENT CONSTRUCTION INDUSTRY STANDARD **BUILDING TECHNIQUES AND** PRACTICES IN THE PRICING AND PRODUCTION OF THIS STRUCTURE. NECESSARY ITEMS NOT SPECIFIED DIRECTLY OR OTHERWISE LISTED AS "NOTE IN CONTRACT" OR "SUPPLIED BY OWNER/OTHERS" ARE TO BE INCLUDED IN THE CONTRACT SUM FOR A COMPLETE "TURN KEY" JOB. THESE ITEMS SHOULD BE BROUGHT TO THE ATTENTION OF THE ARCHITECT UPON DISCOVERY.

OCCUPANCY:		STORAGE - S2
OCCUPANT LOAD:		8 OCCUPANTS
CONSTRUCTION:	TYPE V-B (0	COMBUSTIBLE)
ALLOWABLE HEIGHT:		40'-0"
ALLOWABLE BUILDING AREA:	13,500 S.F. ((UNADJUSTED)
GROSS FLOOR AREA - GROUND	FLOOR:	1,600 S.F.
TOTAL GROSS FLOOR AREA:		1,600 S.F.
NET FLOOR AREA:		N/A
ZONING:	KENSETT	AR - CENTRAL
FIRE SPRINKLER:	No	ONE REQUIRED

INDEX OF DRAWINGS				
Sheet Number	Sheet Name			
T1.0	TITLE SHEET			
AC1.1	SITE PLAN			
S1	FOUNDATION			
S2	FRAMING			
A1.1	FIRST FLOOR PLAN			
A1.3	REFLECTED CEILING PLAN			
A1.4	ROOF PLAN			
A2.1	SOUTH & EAST ELEVATION			
A2.2	NORTH & WEST ELEVATION			
A3.1	BUILDING SECTIONS			
A3.2	BUILDING SECTIONS			
A4.2	EXTERIOR 3D VIEWS			
PME 1	PLUMBING PLAN			
PME 2	ELECTRICAL PLAN			

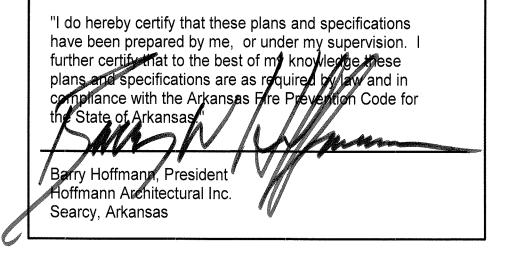


Building Code: 2012 Arkansas Fire Prevention Code

Kensett Fire Department

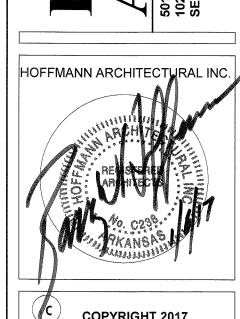
East Searcy Street

Kensett, Arkansas



A PROJECT FOR:

East Searcy Street Kensett, Arkansas



DATE: 01/06/17

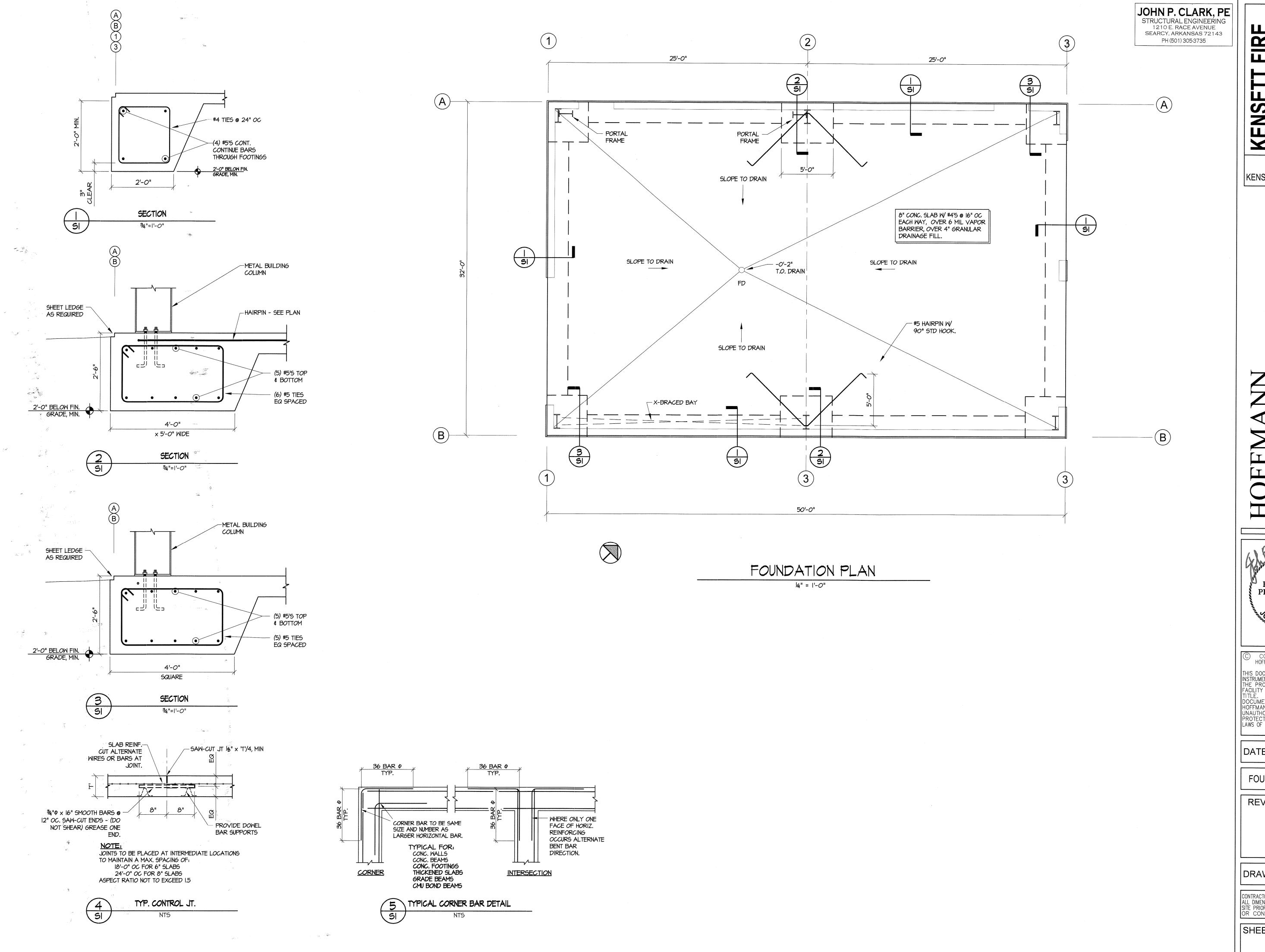
REVISIONS:

CONTRACTOR MUST CHECK AND VERIFY ALL DIMENSIONS AND CONDITIONS ON SITE MATERIALS OR CONSTRUCTION

DRAWN BY: **BWH**

TITLE SHEET

SHEET: T1.0



FIRE KENSET DEPART

KENSETT, ARKANSAS

ARKANSAS REGISTERED PROFESSIONAL **ENGINEER** No.8654

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DATE: 1-6-17

FOUNDATION PLAN

REVISIONS:

DRAWN BY: JPC

CONTRACTOR MUST CHECK AND VERIFY ALL DIMENSIONS AND CONDITIONS ON SITE PRIOR TO ORDERING MATERIALS OR CONSTRUCTION.

S¹

GENERAL NOTES:

- I. THE CONTRACTOR SHALL THOROUGHLY REVIEW ALL CONTRACT DOCUMENTS AND INFORM THE ARCHITECT OF CONFLICTS OR DISCREPANCIES PRIOR TO BIDDING, FABRICATION, AND CONSTRUCTION.
- 2. IN CASES OF DISCREPANCIES IN DIMENSIONS AND ELEVATIONS BETWEEN STRUCTURAL AND ARCHITECTURAL DRAWINGS, CONTRACTOR SHALL COORDINATE WITH THE ARCHITECT PRIOR TO FABRICATION AND CONSTRUCTION.
- 3. THE CONTRACTOR SHALL COORDINATE THE FIELD VERIFICATION OF ALL EXISTING SITE CONDITIONS SUCH AS EXISTING FLOOR ELEVATIONS, EXISTING FOOTING ELEVATIONS, EXISTING UTILITIES, ETC, WHETHER NOTED OR NOT IN THE CONTRACT DOCUMENTS AND SHALL NOTIFY THE ARCHITECT OF ANY CONFLICTS, DISCREPANCIES OR UNKNOWN CONDITIONS PRIOR TO FABRICATION AND CONSTRUCTION.
- 4. REPRODUCTION OF CONTRACT DRAWINGS, IN ANY FORM, WILL NOT BE ACCEPTED AS SHOP DRAWINGS.
- 5. REVIEW OF SUBMITTALS AND/OR SHOP DRAWINGS BY THE STRUCTURAL ENGINEER-OF-RECORD DOES NOT RELIEVE THE CONTRACTOR OF THE RESPONSIBILITY TO REVIEW AND CHECK SHOP DRAWINGS BEFORE SUBMITTAL FOR REVIEW. THE CONTRACTOR REMAINS SOLELY RESPONSIBLE FOR ERRORS AND OMISSIONS ASSOCIATED WITH THE PREPARATION OF SHOP DRAWINGS AS THEY PERTAIN TO MEMBER SIZES, DETAILS AND DIMENSIONS SPECIFIED IN THE CONTRACT DOCUMENTS. CONTRACTOR ALSO SHALL BE RESPONSIBLE FOR ALL MEANS, METHODS, TECHNIQUES, AND PROCEDURES OF CONSTRUCTION.
- 6. CONTRACTOR SHALL PROVIDE TEMPORARY GUYS AND BRACING AS REQUIRED DURING CONSTRUCTION. STRUCTURE IS NOT STABLE UNTIL ALL STRUCTURAL MEMBERS, CONNECTIONS, AND DECKING IS IN PLACE.
- 7. ACI, AISC, AITC AND AWS SPECIFICATIONS SHALL GOVERN ALL PHASES OF FABRICATION AND CONSTRUCTION.

FOUNDATION NOTES

- I. CONCRETE SHALL BEAR ON FIRM NATURAL SOIL OR PROPERLY COMPACTED SELECT FILL WITH AN ALLOWABLE NET BEARING CAPACITY OF AT LEAST 1800 PSF.
- 2. ALL FOOTING EXCAVATIONS SHALL BE INSPECTED BY THE SOILS ENGINEER OR HIS REPRESENTATIVE TO VERIFY THAT THE DESIGN NET BEARING CAPACITY IS ATTAINABLE.
- 3. SOFT SOILS ENCOUNTERED BENEATH THE BUILDING SHALL BE UNDERCUT AND REPLACED WITH PROPERLY COMPACTED, LOW PLASTICITY, SELECT FILL. BACKFILL BOTH SIDES OF ALL WALLS EQUALLY UNTIL LOW SIDE IS UP TO GRADE.
- 4. ALL FILL SHALL BE COMPACTED TO AT LEAST 95% OF MODIFIED PROCTOR MAXIMUM DRY DENSITY, ASTM
- (5. FOOTING DESIGN SHOWN ON THESE DRAWING ARE FOR ESTIMATING PURPOSES & ARE SUBJECT TO REVISIONS PENDING REVIEW OF METAL BUILDING DESIGN REACTIONS & SOIL ENGINEER'S REPORT.

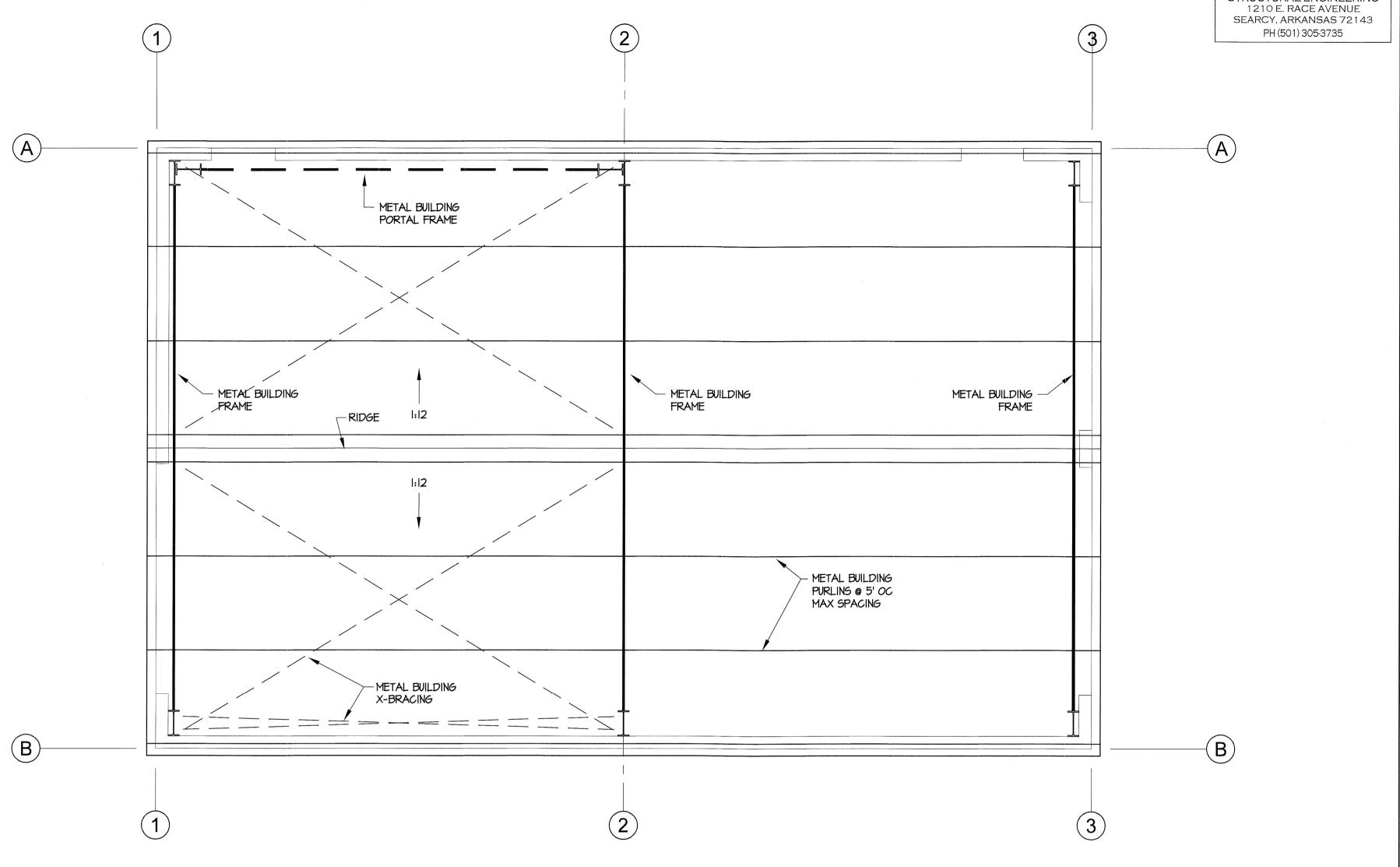
 CONTRACTOR TO SUBMIT METAL BUILDING REACTIONS TO ARCHITECT FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION. FINAL FOOTING SIZES MAY VARY FROM SIZES INDICATED ON THESE DRAWINGS.

CONCRETE:

- I. CONCRETE MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS SHALL BE 3,500 PSI.
- 2. ALL MILD REINFORCEMENT BAR SHALL BE A615 GRADE 60 STEEL. ALL WELDED WIRE FABRIC SHALL CONFORM TO ASTM A185, GRADE 65. ALL WELDED WIRE FABRIC SHALL BE IN SHEETS AND SHALL BE SUPPORTED ON CHAIRS.
- 3. BENDING DIMENSIONS & TOLERANCES FOR REINFORCING BAR SHALL CONFORM TO CURRENT CRSI MANUAL. OF STANDARD PRACTICE.
- 4. LAP SPLICES SHALL CONFORM TO THE CURRENT CRSI MANUAL OF STANDARD PRACTICE UNLESS OTHERWISE NOTED.
- 5. HORIZONTAL CONSTRUCTION JOINTS TO BE SCRUBBED WITH A COARSE WIRE BRUSH AT THE APPROXIMATE TIME OF INITIAL SET TO REMOVE ALL LATINATE AND TO PRODUCE A ROUGHENED SURFACE.
- 6. CONCRETE WORK SHALL COMPLY WITH THE LATEST ACI "SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS (ACI 301) AND APPLICABLE PROVISIONS OF ACI 318, KEEP A COPY OF ACI FIELD REFERENCE MANUAL (ACI SP-15) WHICH INCLUDES ACI 301 AND OTHER ACI AND ASTM REFERENCES ON THE JOB.
- 7. DETAILING, FABRICATING, AND PLACING OF REINFORCING STEEL AND ACCESSORIES SHALL BE IN ACCORDANCE WITH ACI "MANUAL OF STANDARD PRACTICE FOR REINFORCED CONCRETE STRUCTURES" (ACI 315) AND SHALL COMPLY WITH (ACI 316) AND WITH (ACI 301).

PRE-ENGINEERED METAL BUILDING:

- I. METAL BUILDING MANUFACTURER SHALL PROVIDE SHOP DRAWINGS SEALED AND SIGNED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF ARKANSAS TO THE ARCHITECT FOR REVIEW PRIOR TO FABRICATION.
- 2. METAL BUILDING SHOP DRAWINGS WILL NOT BE REVIEWED IF THE LAYOUT DOES NOT FOLLOW THE LAYOUT PROPOSED IN THE CONTRACT DRAWINGS AND IF ANY DEVIATIONS FROM THE PROPOSED LAYOUT ARE NOT CLEARLY MARKED ON THE SHOP DRAWINGS OR APPROVED IN WRITING PRIOR TO SUBMITTAL.
- 3. METAL BUILDING FRAMING LAYOUT AND MEMBERS SHOWN ARE SUGGESTED ONLY. MANUFACTURER IS RESPONSIBLE FOR COORDINATING REQUIREMENTS WITH OWNER AND PROVIDING A COMPLETE STRUCTURAL FRAMING SYSTEM DESIGNED BY THE MANUFACTURER. METAL BUILDING MANUFACTURER SHALL COORDINATE ALL DIMENSIONS, ELEVATIONS, BRACING, AND SIZES AND SHAPES OF MEMBERS WITH OWNER PRIOR TO FABRICATION AND CONSTRUCTION. ALL MEMBERS, CONNECTIONS AND DECKING NOT SPECIFICALLY SIZED ON DRAWINGS SHALL BE DESIGNED AND SUPPLIED BY THE METAL BUILDING MANUFACTURER.
- 4. METAL BUILDING VERTICAL BRACING SHALL CONSIST OF PORTAL FRAMES OR ROD BRACES AT THE LOCATIONS SHOWN ON THE PLANS. THE METAL BUILDING MANUFACTURER SHALL COORDINATE THE LOCATION OF ALL BRACES TO MINIMIZE INTERFERENCE WITH ARCHITECTURAL FEATURES. ROD OR CABLE BRACES MAY NOT BE SUBSTITUTED WHERE PORTAL FRAMES ARE SHOWN. WHERE X-BRACES ARE USED, THE METAL BUILDING MANUFACTURER SHALL CLEARLY IDENTIFY TO THE ARCHITECT ALL INTERFERENCES WITH ARCHITECTURAL FEATURES. WHERE ARCHITECTURAL FEATURES (COLUMN SURROUNDS, CEILINGS, FURR DOWNS, ETC) ARE PROVIDED TO COVER OR SURROUND THE METAL BUILDING COMPONENTS (COLUMNS, FRAMES, ETC.), THE METAL BUILDING COMPONENTS SHALL BE SIZED TO STAY WITHIN THE LIMITS OF THE ARCHITECTURAL FEATURES UNLESS THE ARCHITECT IS NOTIFIED IN WRITING PRIOR TO SUBMISSION OF THE APPROVAL DRAWINGS AND APPROVAL IS GIVEN FOR AN EXCEPTION.
- 5. MAXIMUM PURLIN LIVE LOAD DEFLECTION SHALL NOT EXCEED SPAN/360 OR I" WHICHEVER IS LESS.
- 6. FRAME LIVE LOAD DEFLECTION SHALL NOT EXCEED SPAN/360.
- 7. MAXIMUM GIRT LATERAL DEFLECTION FROM WIND OR SEISMIC LOADS SHALL NOT EXCEED SPAN/240 FOR GIRTS PROVIDING LATERAL SUPPORT FOR METAL SIDING ONLY. MAXIMUM GIRT LATERAL DEFLECTION FROM WIND OR SEISMIC LOADS SHALL NOT EXCEED SPAN/360 FOR GIRTS PROVIDING LATERAL SUPPORT FOR BRICK.
- 8. MAXIMUM BUILDING SIDESWAY (DRIFT) FROM WIND OR SEISMIC LOADS SHALL NOT EXCEED WALL HEIGHT/67.
- 9. THE GENERAL CONTRACTOR AND METAL BUILDING MANUFACTURER SHALL BE RESPONSIBLE FOR OVERALL BUILDING COORDINATION. ALL COORDINATION OF THE INTERFACE AND COMPATIBILITY BETWEEN THE METAL BUILDING AND THE ARCHITECTURAL FEATURES SHALL BE THE RESPONSIBILITY OF THE GENERAL CONTRACTOR AND THE METAL BUILDING MANUFACTURER.
- IO. DESIGN OF THE METAL BUILDING USING DEAD, LIVE, SEISMIC, WIND AND SNOW LOADS IN THE CODE REQUIRED COMBINATIONS SHALL BE PERFORMED BY THE METAL BUILDING MANUFACTURER.





DESIGN LOADS - IBC 2012 \$ ASCE 7 - 10:

WEIGHT OF THE STRUCTURE

20 PSF

II. ANALYSIS PROCEDURE : EQUIVALENT LATERAL FORCE PROCEDURE

DEAD LOADS:

ROOF LIVE LOAD: FLOOR LIVE LOADS: ROOF FRAMING PLAN

1/4" = 1'-0"

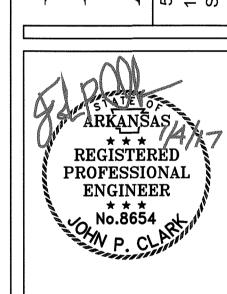
SLEEPING ROOMS 40 PSF **CLASSROOMS** 40 PSF CELL BLOCKS 40 PSF 50 PSF OFFICES PARKING GARAGES 50 PSF LIBRARY READING ROOMS 60 PSF CORRIDORS 80 PSF 100 PSF LOBBIES ASSEMBLY AREAS 100 PSF STAIRS 100 PSF BLEACHERS/GRANDSTANDS 100 PSF MECH MEZZANINE 60 PSF STAGES 125 PSF LIBRARY STACK ROOMS 125 PSF PRE-ENGINEERED METAL BUILDING LOADS: <u>GROUND SNOW LOAD:</u> DEAD LOADS: WEIGHT OF THE STRUCTURE FLAT ROOF SNOW LOAD, PF: IO PSF 20 PSF (NO REDUCTIONS ALLOWED) ROOF LIVE LOAD: SNOW EXPOSURE FACTOR, Ce: 1.2 COLLATERAL LOAD: HANGING EQUIP'T, LIGHTS, CEILINGS, SNOW IMPORTANCE FACTOR, I: THERMAL FACTOR, Ct ETC. (7 PSF MINIMUM COLLATERAL LOAD. INCLUDE ACTUAL WT OF SUSPENDED EQ'T.) I. ULTIMATE WIND SPEED, Vult: 120 MPH ALSO SEE 'DESIGN LOADS' LISTED ON THIS SHEET FOR NOMINAL WIND SPEED, Vasd: 93 MPH ADDITIONAL REQUIREMENTS. 2. RISK CATEGORY: IV 3. WIND EXPOSURE : B 4. INTERNAL PRESSURE COEFFICIENT(S): GCPI = +/- O.18 \$ +/- O.55 RISK CATEGORY: IV 2. SEISMIC IMPORTANCE FACTOR, Ie = 1.5 3. MAPPED SPECTRAL RESPONSE: 56 = 0.619I HEREBY CERTIFY THAT THE STRUCTURAL PLANS SUBMITTED HEREWITH 51 = 0.228ARE DESIGNED WITH THE STRUCTURAL LOAD CARRYING ELEMENTS TO 4: SITE CLASS: D RESIST THE ANTICIPATED FORCES OF THE DESIGNATED SEISMIC ZONE IN 5. DESIGN SPECTRAL RESPONSE FACTORS: WHICH THE STRUCTURE IS LOCATED IN ACCORDANCE WITH ARKANSAS 505 = 0.538ACT 1100 OF 1991. SDI = 0.2966. SEISMIC DESIGN CATEGORY: D METAL BUILDING: 7. BASIC SEISMIC RESISTING SYSTEM: C. MOMENT RESISTING FRAMES I. STEEL ORDINARY MOMENT FRAMES 8. DESIGN BASE SHEAR, V=0.231 x W 9. SEISMIC RESPONSE COEF, Cs. O.231 JOHN P. CLARK, P.E. # 8654 IO RESPONSE MODIFICATION FACTOR, R = 31/2

KENSETT FIRE DEPARTMENT

JOHN P. CLARK, PE

KENSETT, ARKANSAS

FMANN



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ROOF FRAMING

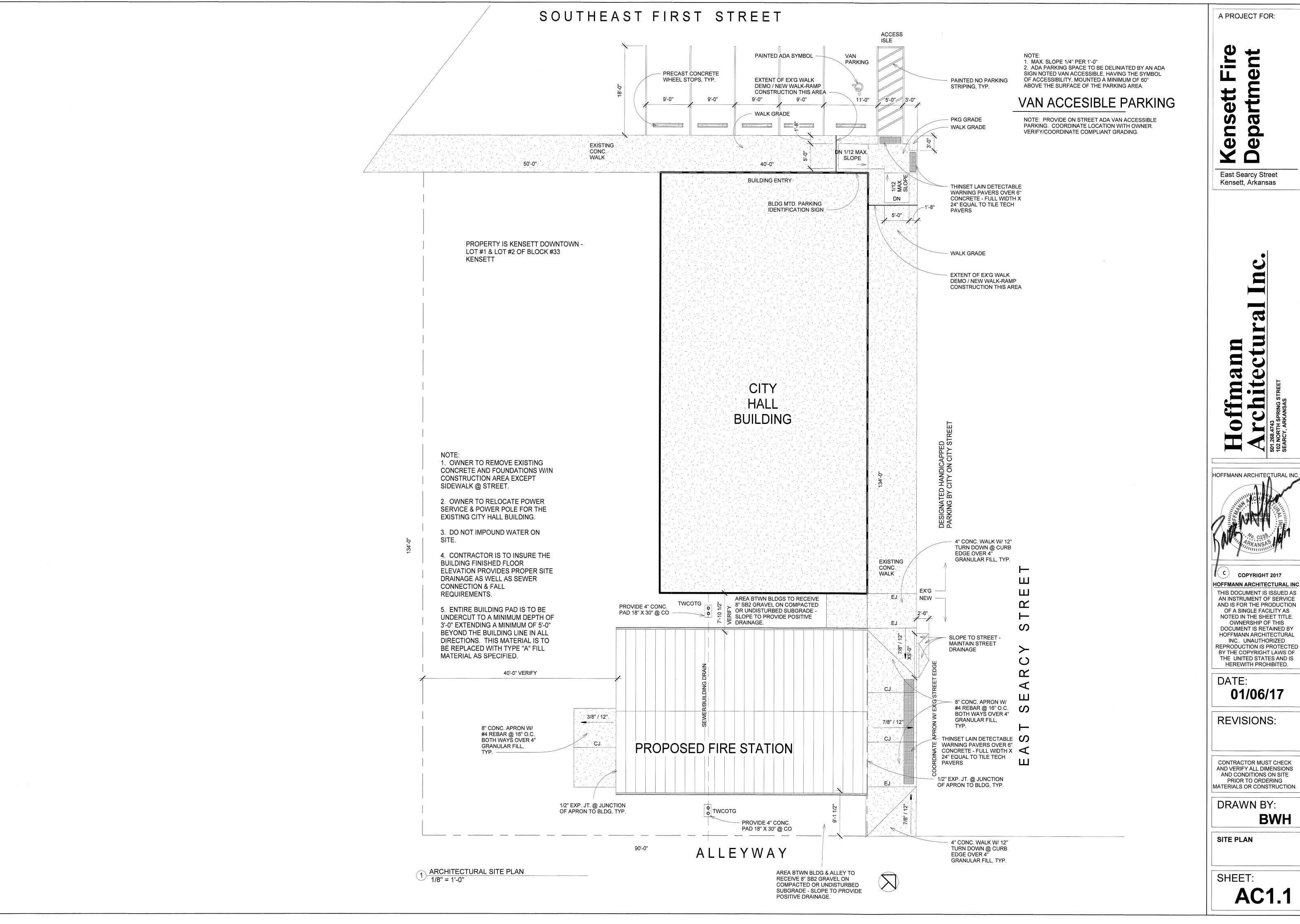
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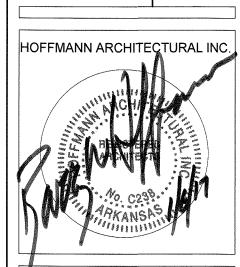
UZ

SHEET:



rtment

East Searcy Street



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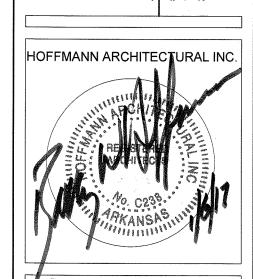
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AC1.1

	DOOR SCHEDULE						
Mark	WDTH	HG T	THKNS	Frame Material	Construction Type	HARDWARE	Comments
101A	3' - 0"	6' - 8"	0' - 1 3/4"	HOLLOW METAL	HOLLOW METAL	KEYED LOCKSET W/ CLOSER EQUAL TO SARGENT 65G05 LOCKSET, SARGENT 484 DEADBOLT, W/ SARGENT 1431 CLOSER	
101B	3' - 0"	6' - 8"	0' - 1 3/4"	HOLLOW METAL	HOLLOW METAL	KEYED LOCKSET W/ CLOSER EQUAL TO SARGENT 65G05 LOCKSET, SARGENT 484 DEADBOLT, W/ SARGENT 1431 CLOSER	
101C	12' - 0"	14' - 0"	0' - 1 1/2"		EQUAL TO CLOPAY		POWER OPERATED, FLUSH PANEL, 7 SECTION, 2" POLYURETHANE INSULATED STEEL
101D	12' - 0"	14' - 0"	0' - 1 1/2"		EQUAL TO CLOPAY		POWER OPERATED, FLUSH PANEL, 7 SECTION, 2" POLYURETHANE INSULATED STEEL
101E	12' - 0"	14' - 0"	0' - 1 1/2"		EQUAL TO CLOPAY		POWER OPERATED, FLUSH PANEL, 7 SECTION, 2" POLYURETHANE INSULATED STEEL
102	3' - 0"	6' - 8"	0' - 1 3/4"	HOLLOW METAL	HOLLOW METAL	PRIVACY LOCKSET W/ CLOSER EQUAL TO SARGENT 65U65 LOCKSET, W/ SARGENT 1431 CLOSER	

	ROOM FINISH SCHEDULE						
Number	Name	Floor Finish	Base Finish	Wall Finish	Ceiling Finish	Area	Comments
101	TRUCK BAY	SEALED CONCRETE USING PRESOCO SLX100 OR EQUAL	4" RUBBER COVE @ GYP. BD./NONE @ MTL BLDG.		SIMPLE SAVER INSULATION SYSTEM OR EQUAL	1439 SF	NO BASE OR PAINT @ MTL. BUILDING SYSTEM
102	TOILET	SEALED CONCRETE USING PRESOCO SLX100 OR EQUAL	4" RUBBER COVE	PAINT @ GYP. BD COLOR TO BE DETERMINED	SIMPLE SAVER INSULATION SYSTEM OR EQUAL	40 SF	CEILING & DECK TO PAINT TYPICAL

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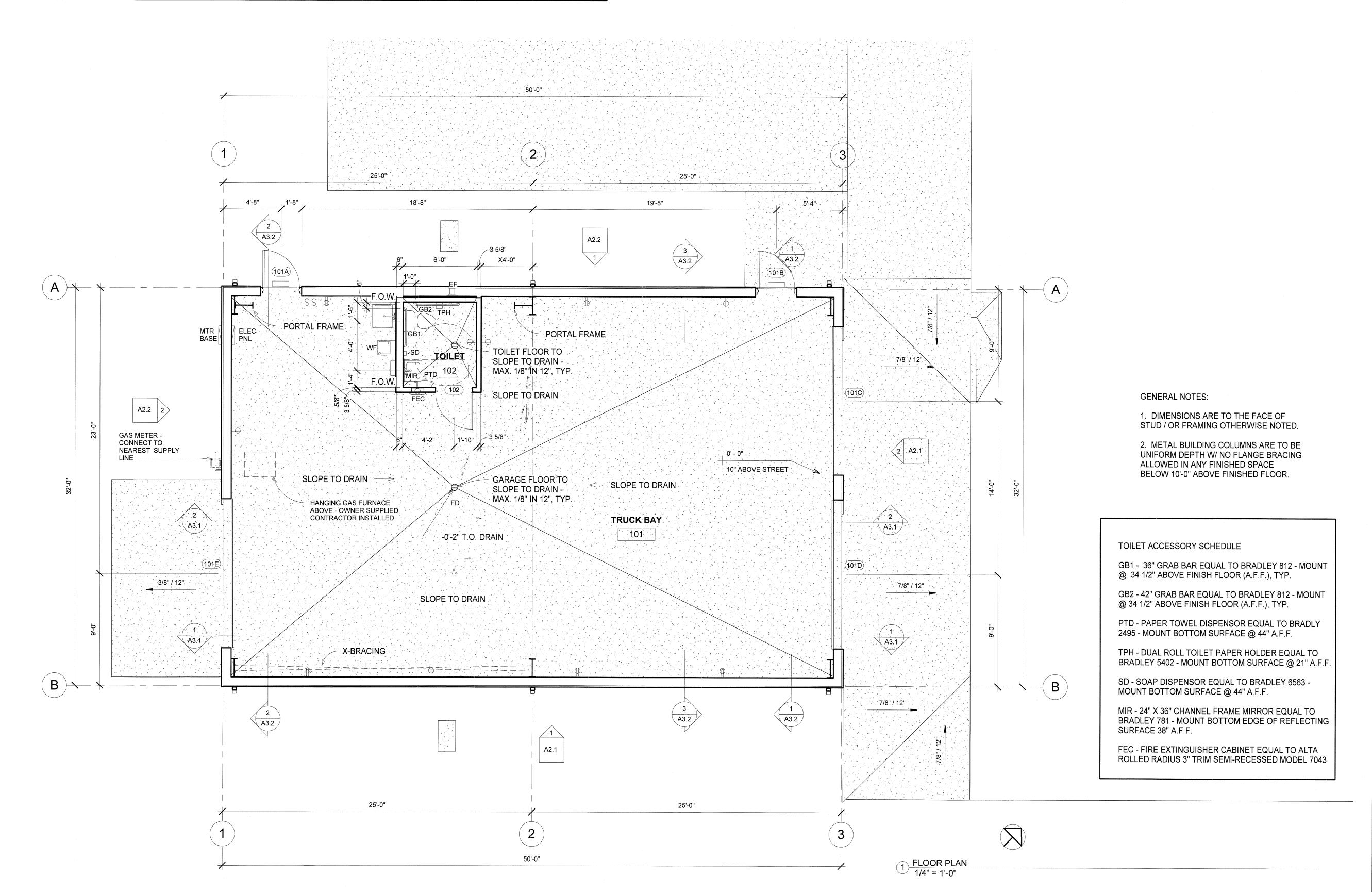
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FIRST FLOOR PLAN

SHEET:

A1.1

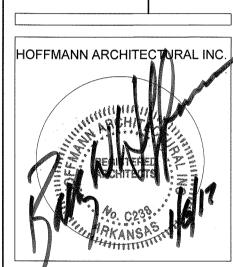


1ST FLOOR REFLECTED CEILING PLAN
1/4" = 1'-0"

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REFLECTED CEILING PLAN

SHEET:

A1.3

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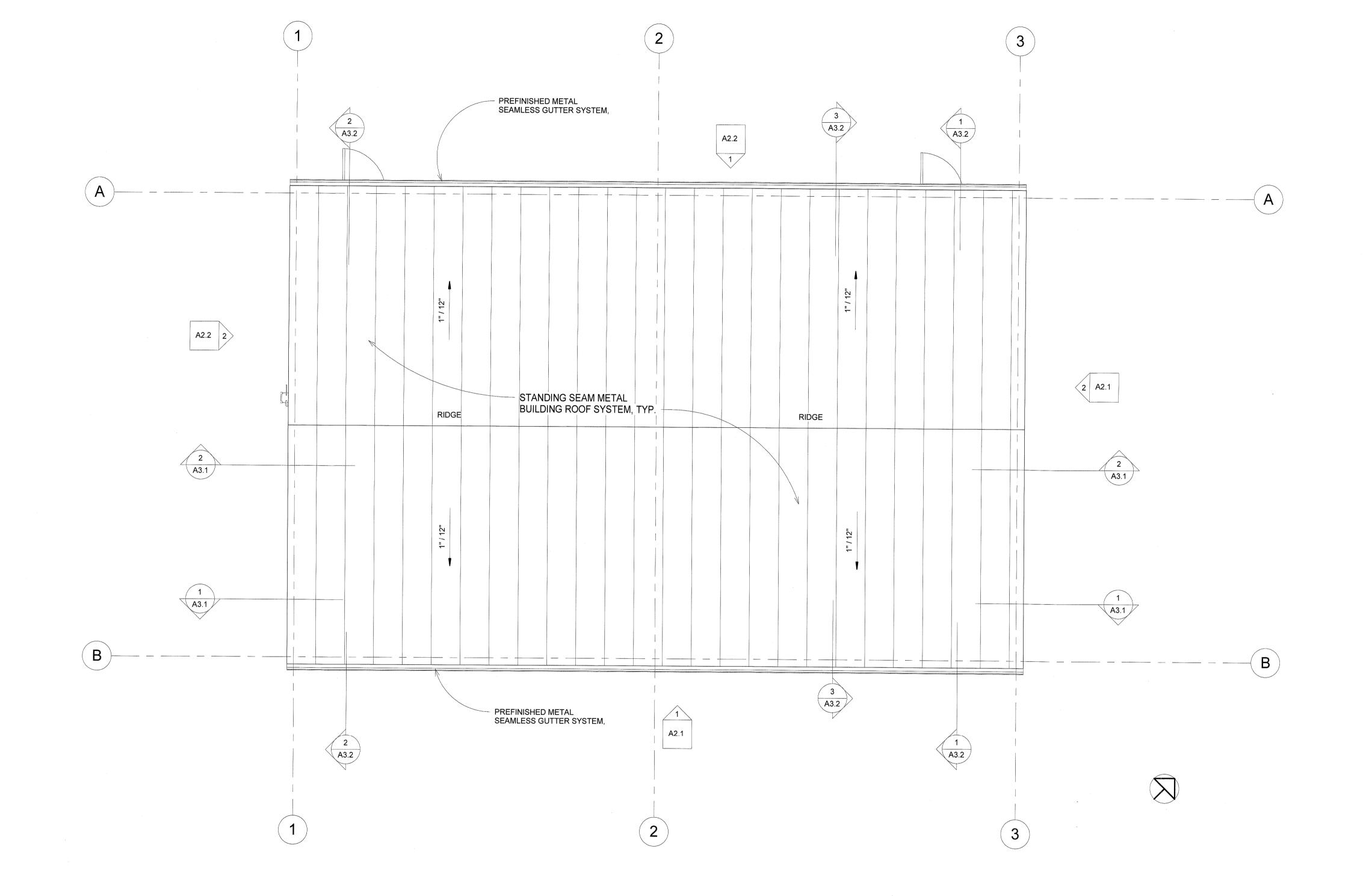
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ROOF PLAN

SHEET: **A1.4**



1 ROOF PLAN 1/4" = 1'-0"

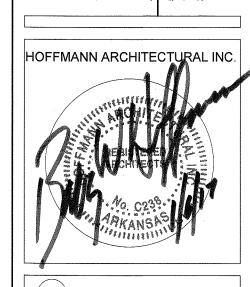
2 EASTERN ELEVATION 1/4" = 1'-0"

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BWH

SOUTH & EAST ELEVATION

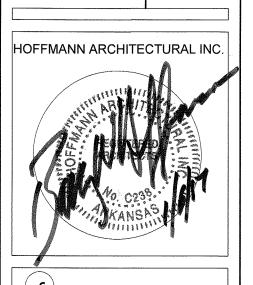
SHEET:

A2.1

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BWH

NORTH & WEST ELEVATION

SHEET:

A2.2

STANDING SEAM METAL A3.2 BATT INSULATION SYSTEM EQUAL TO SIMPLE SAVER 8" R-25 A3.2 BUILDING ROOF SYSTEM 3 A3.2 EAVE HEIGHT 16' - 0" EAVE HEIGHT 16' - 0" ----------HANGING GAS FURNACE ABOVE - OWNER SUPPLIED, CONTRACTOR INSTALLED 5/8" WATER RESISTANT GYPSUM BOARD FOR PAINT, TYP. TRUCK BAY 101 101A 101B 102 FEC - EXPOSED METAL BLDG FRAME & WALL GIRTS, TYP. FLOOR PLAN
0' - 0" FLOOR PLAN 0' - 0" SEE STRUCTURAL FOR FOUNDATION-SIZING & DETAILS

A PROJECT FOR:

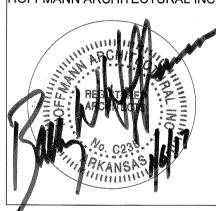
Department Kensett

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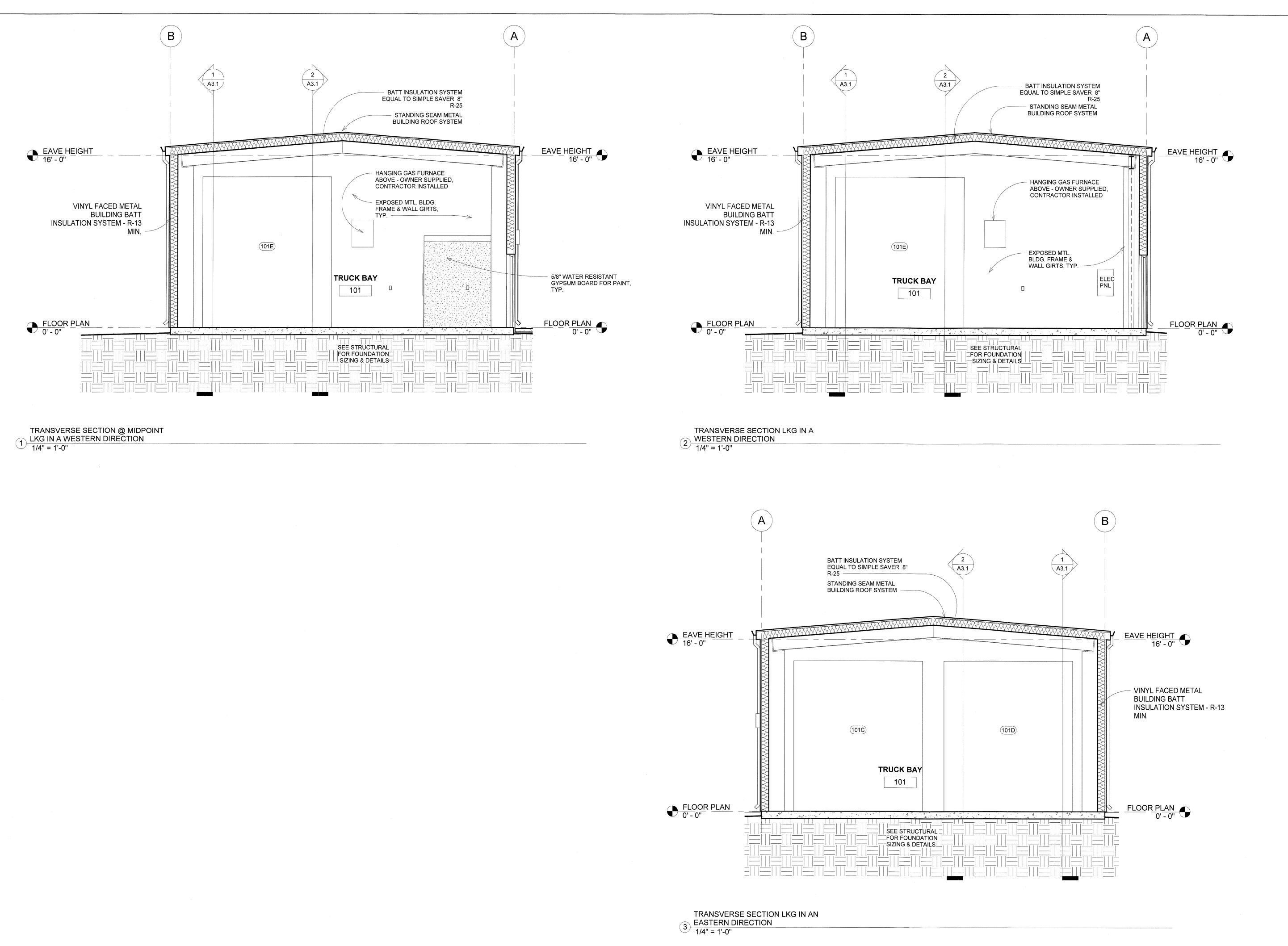
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BUILDING SECTIONS

SHEET:

A3.1

LONGITUDINAL SECTION LKG IN 2 NORTHERN DIRECTION
1/4" = 1'-0"



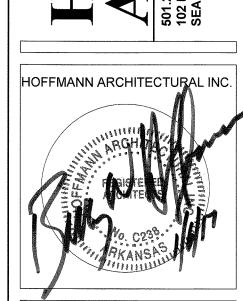
Fire Department Kensett

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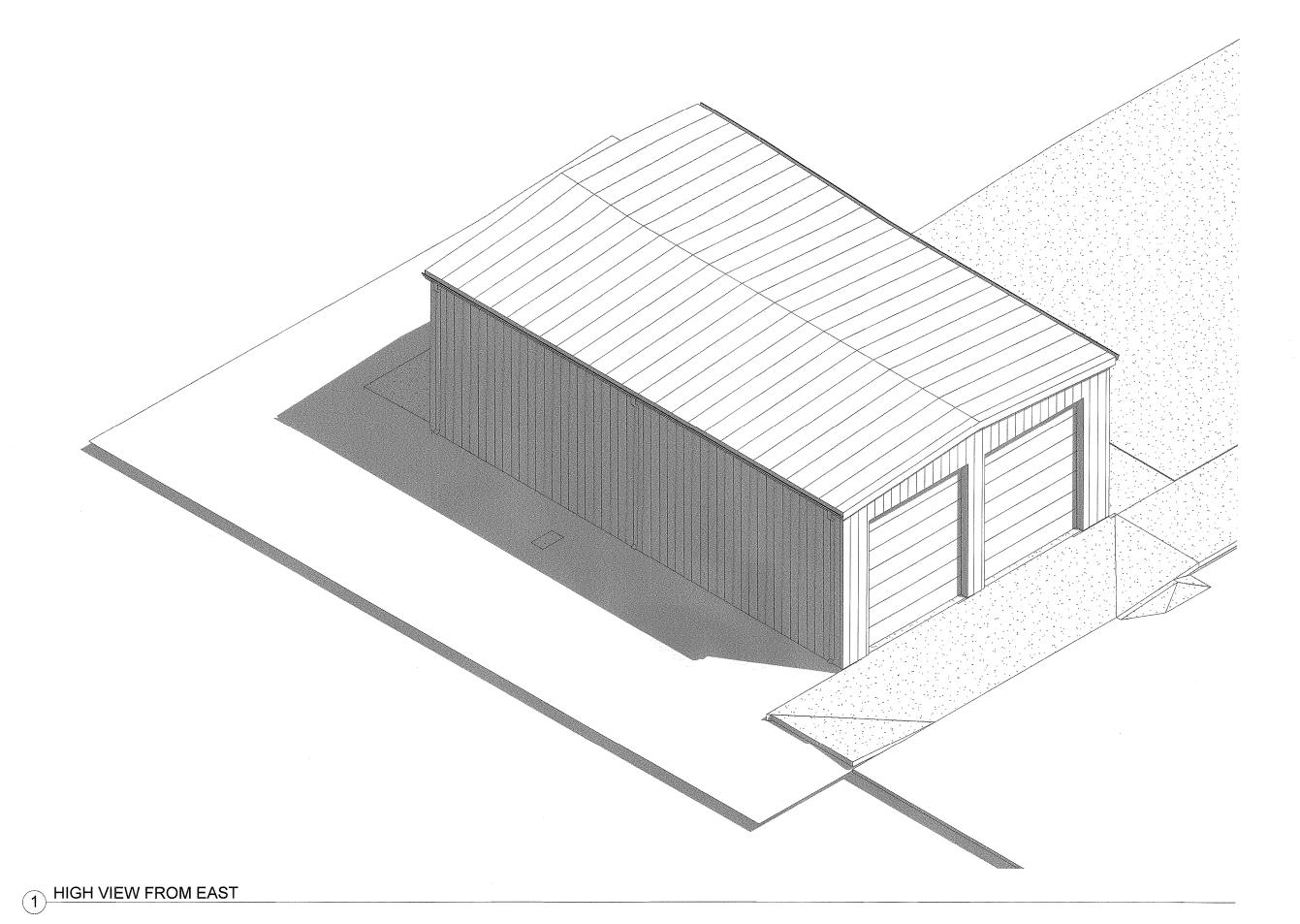
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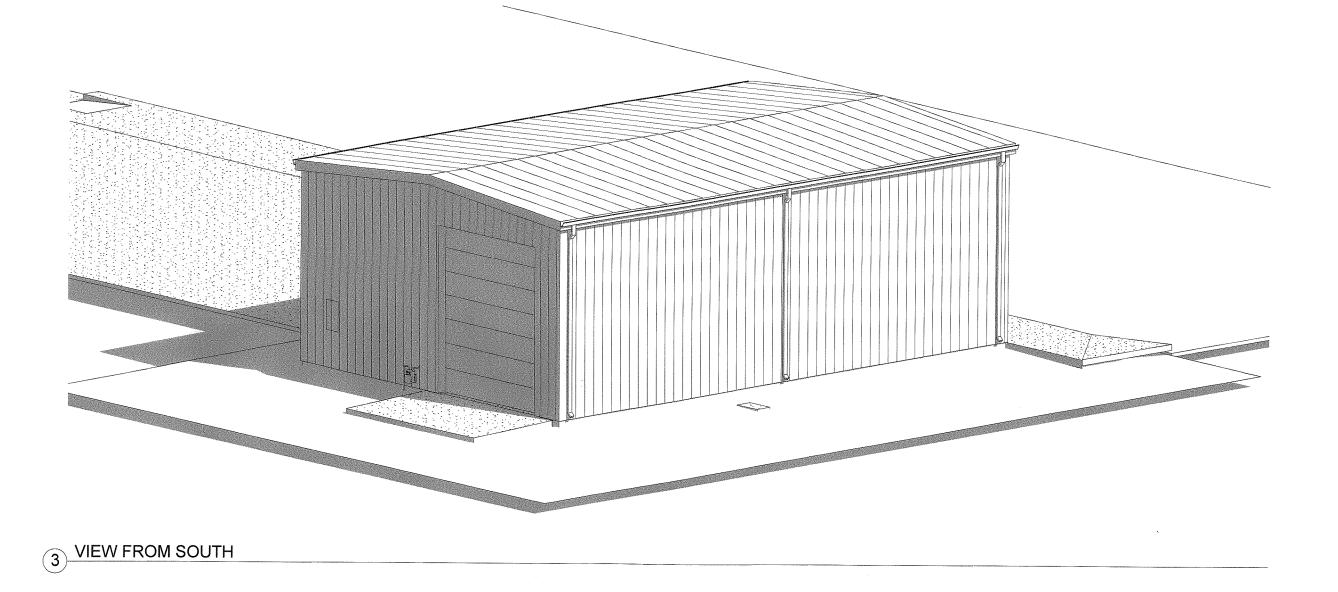
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BUILDING SECTIONS

SHEET:

A3.2





VIEW FROM THE NORTH

A PROJECT FOR:

Kensett Fire Department

East Searcy Street Kensett, Arkansas

Hoffmann Architectural Inc. 601.268.4743 102.10RTH SPRING STREET

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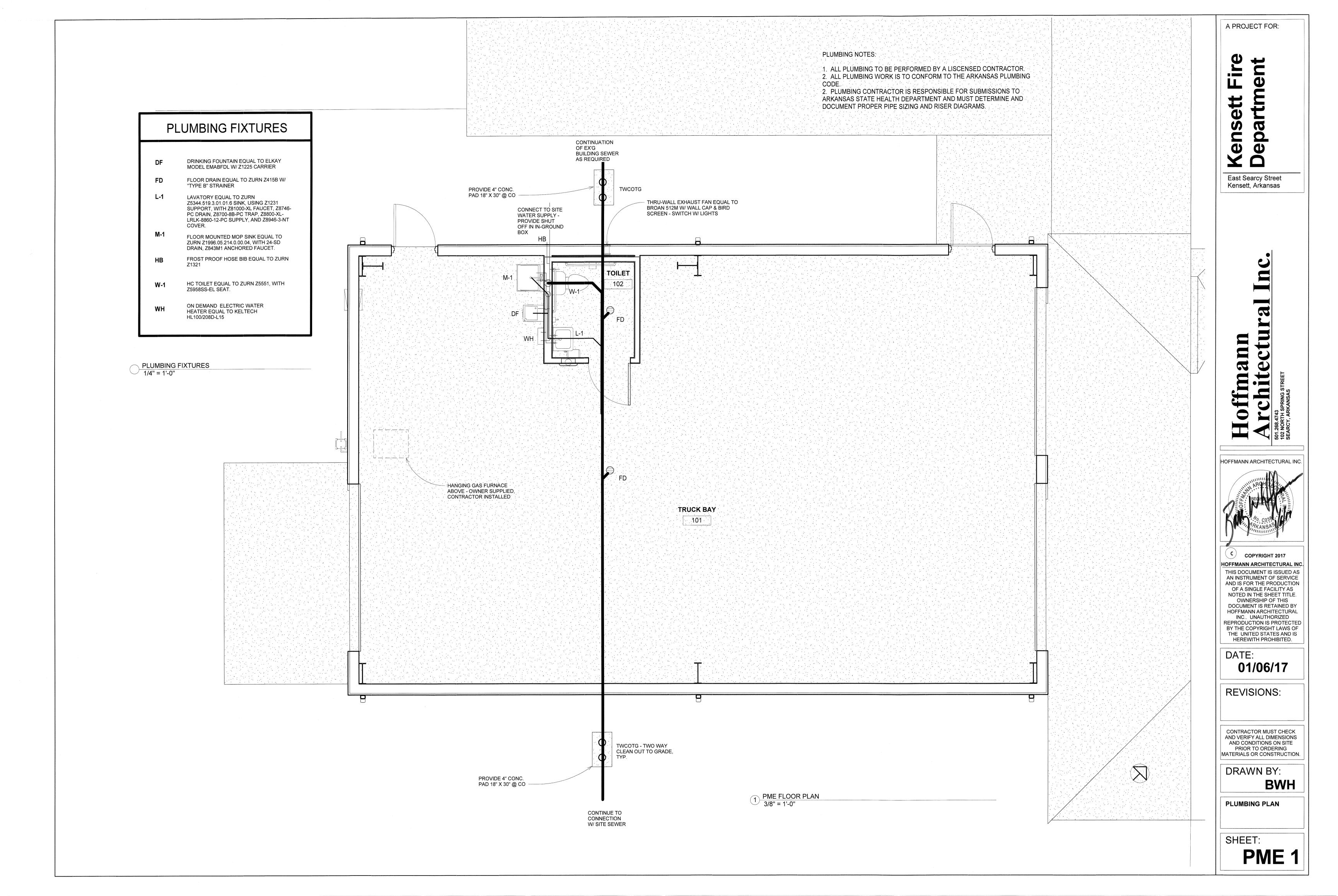
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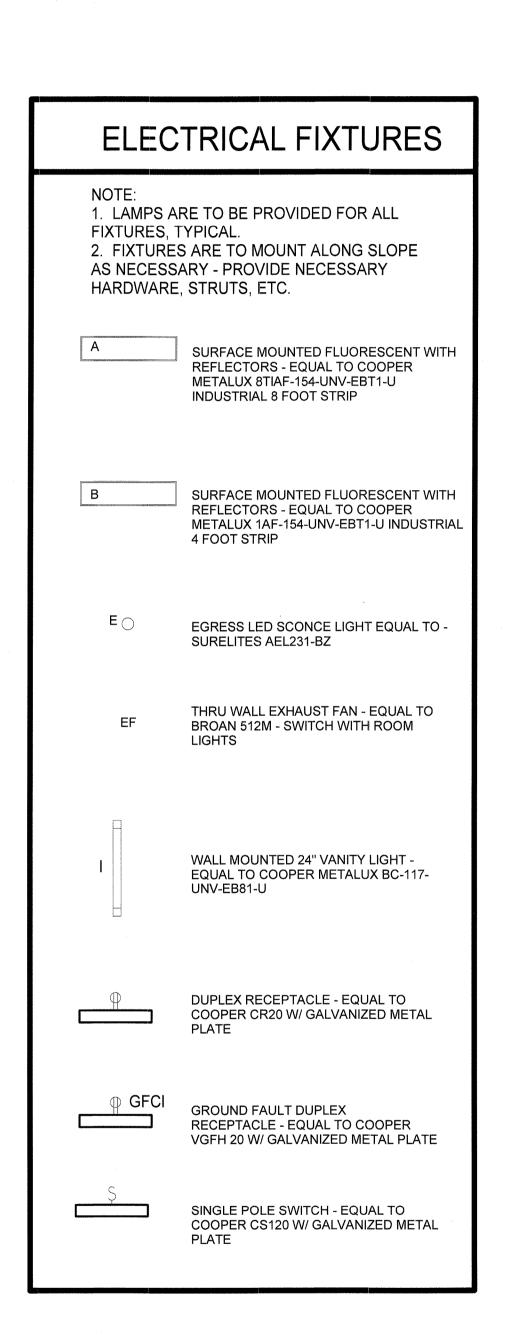
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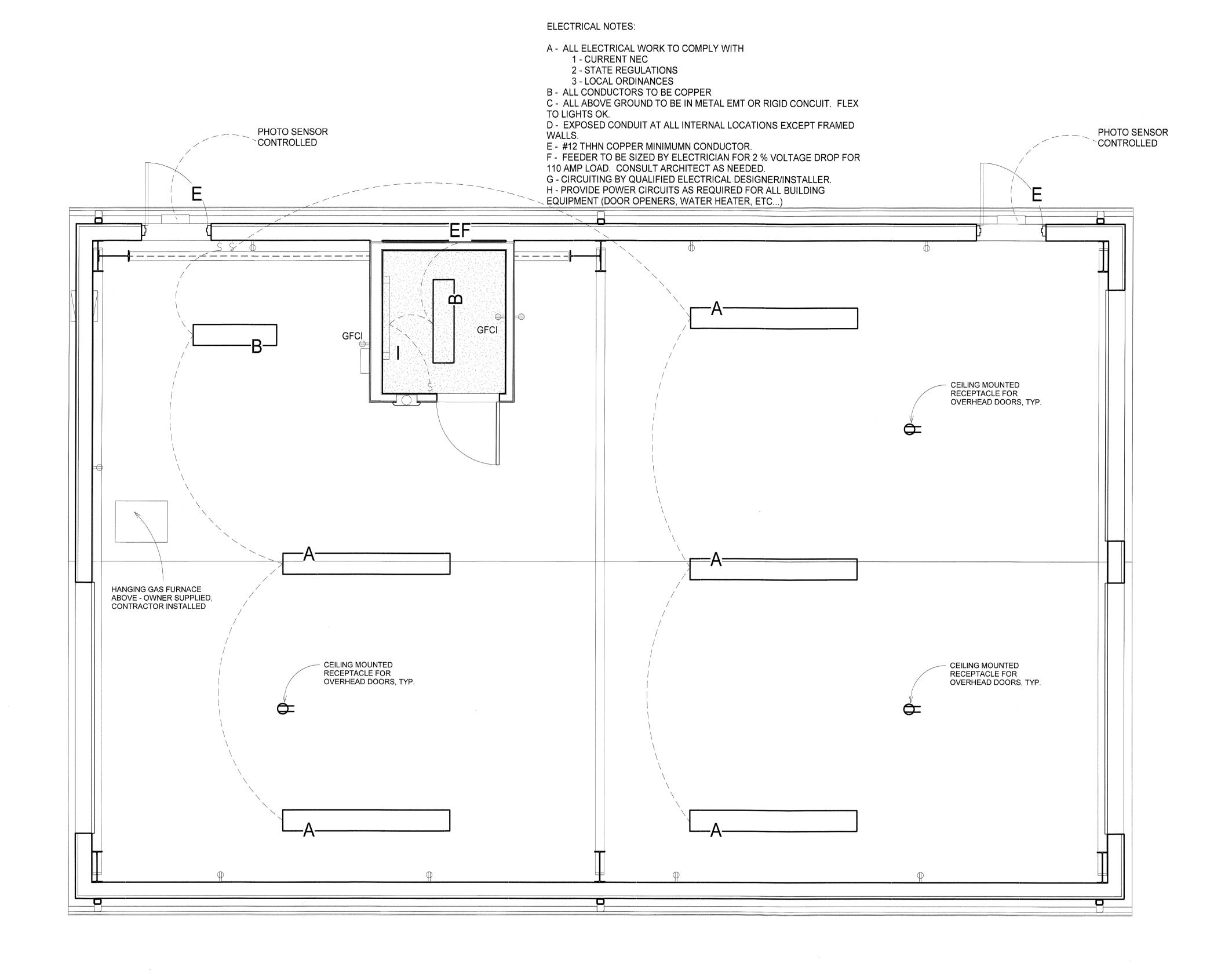
EXTERIOR 3D VIEWS

SHEET:

A4.2







ELECTRICAL FIXTURES
1/4" = 1'-0"

1 ELECTRICAL PLAN 3/8" = 1'-0" A PROJECT FOR:

Sensett Fire

East Searcy Street

Kensett, Arkansas

Hoffmann Architectural Ind

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ELECTRICAL PLAN

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PME 2