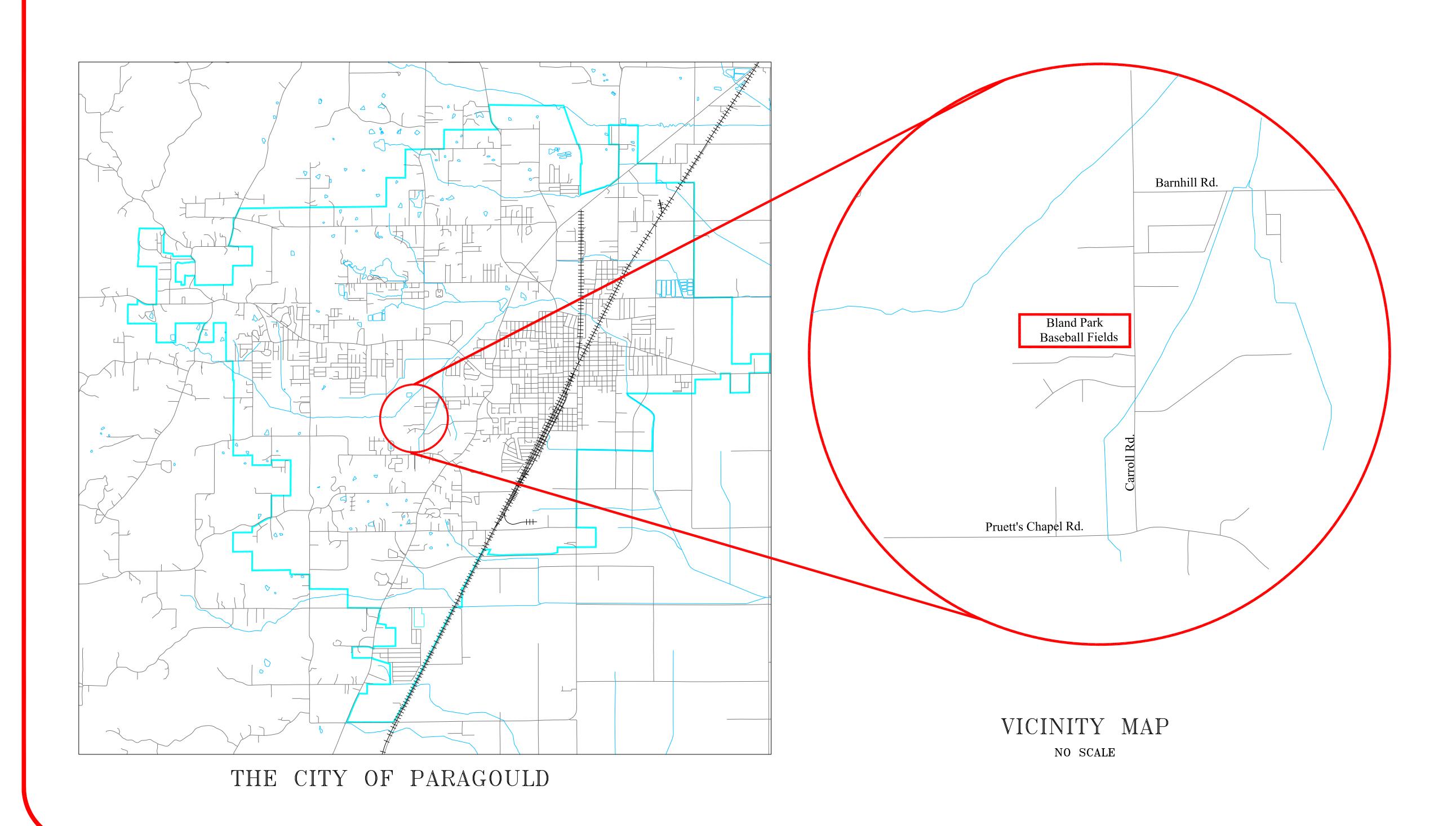
## CITY OF PARAGOULD

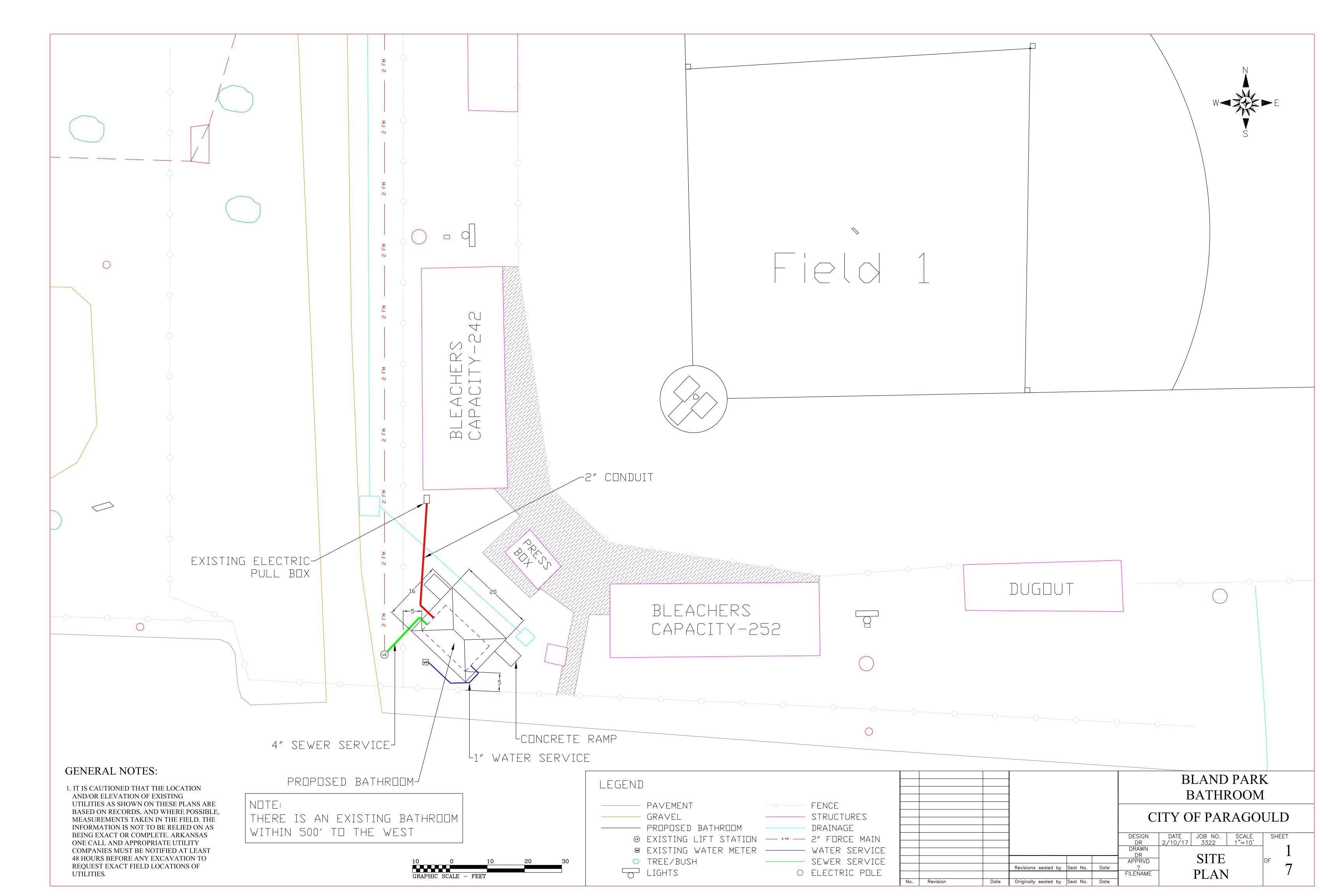
## BLAND PARK BATHROOM

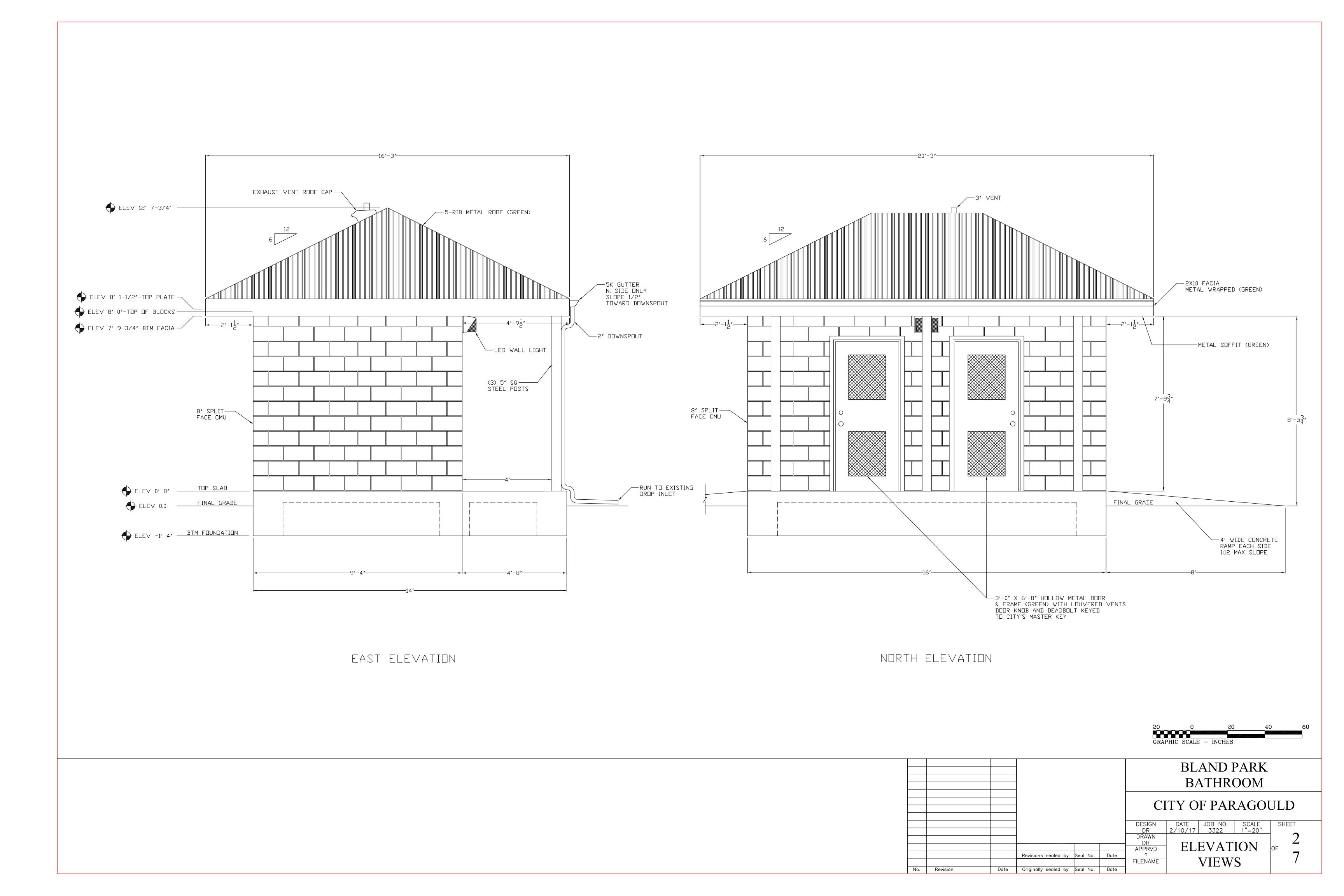
Job Number: 3322

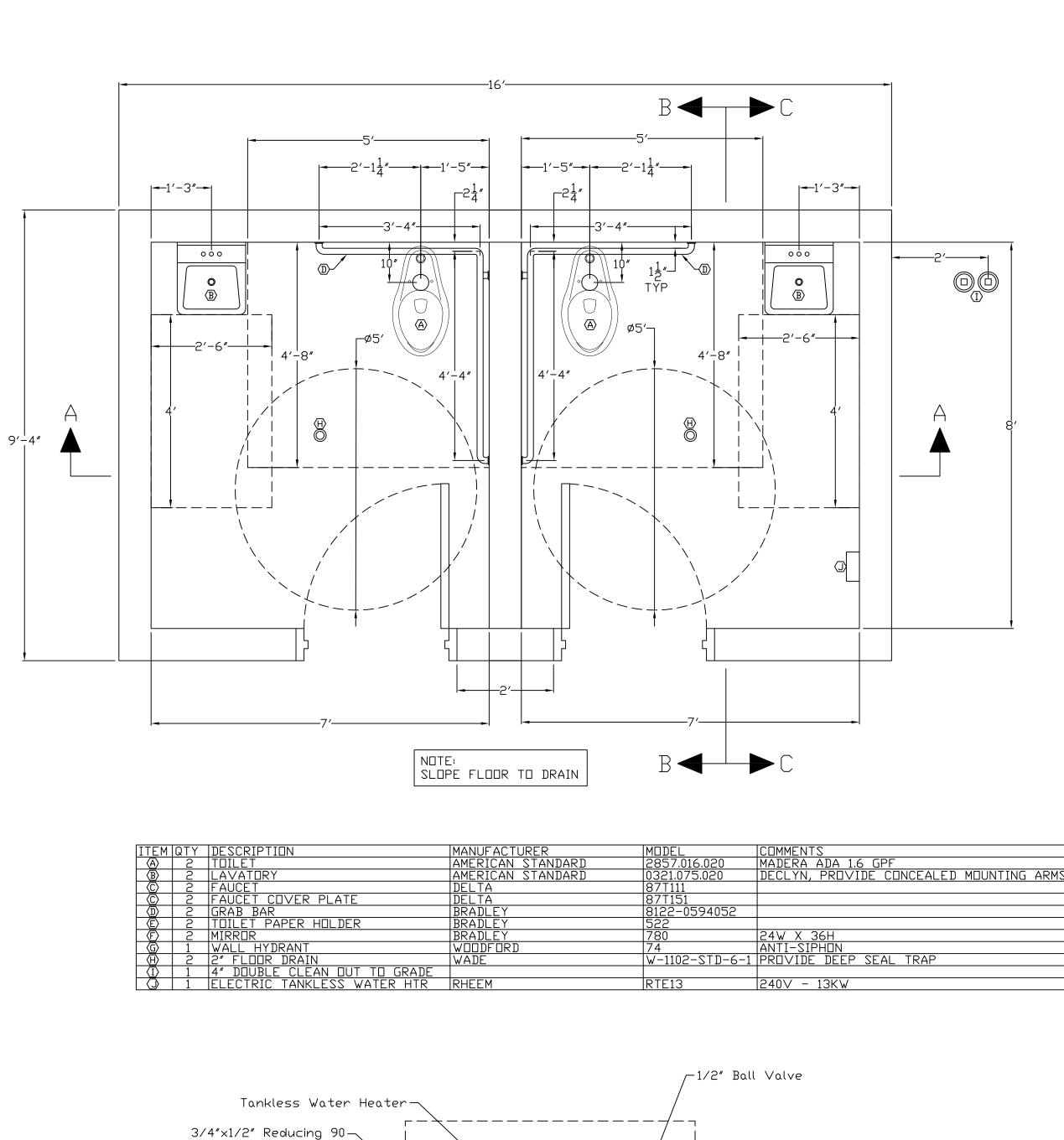


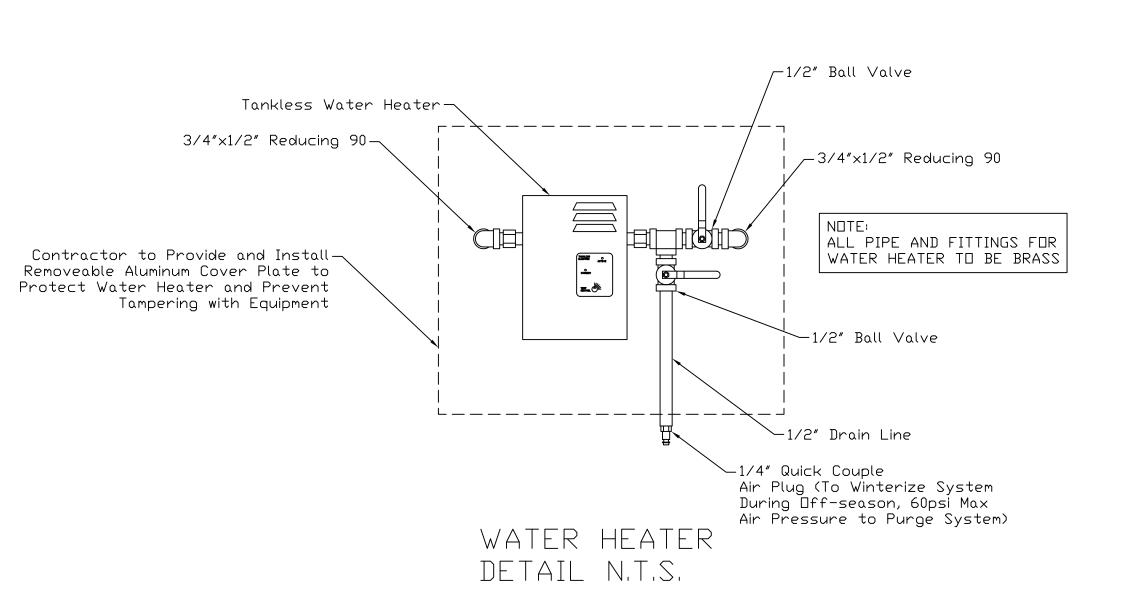
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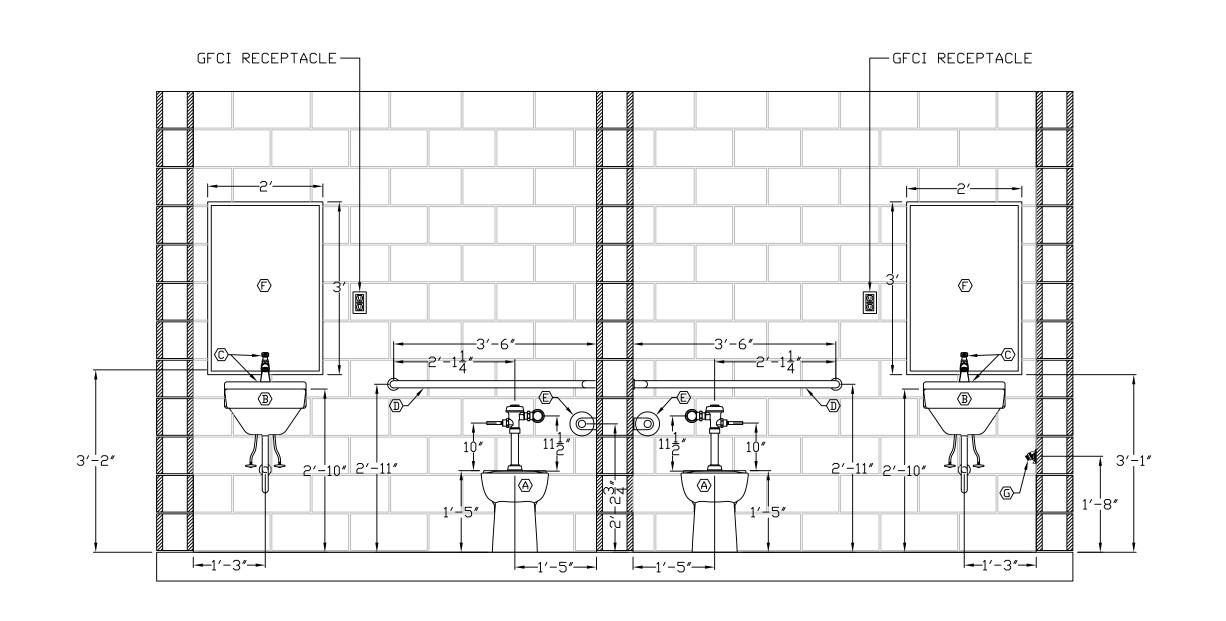
TITLE
COVER SHEET
SITE PLAN
ELEVATION
FLOOR PLAN
FOUNDATION PLAN
ROOF PLAN
ELECTRIC/PLUMBING
<b>SPECIFICATION</b>



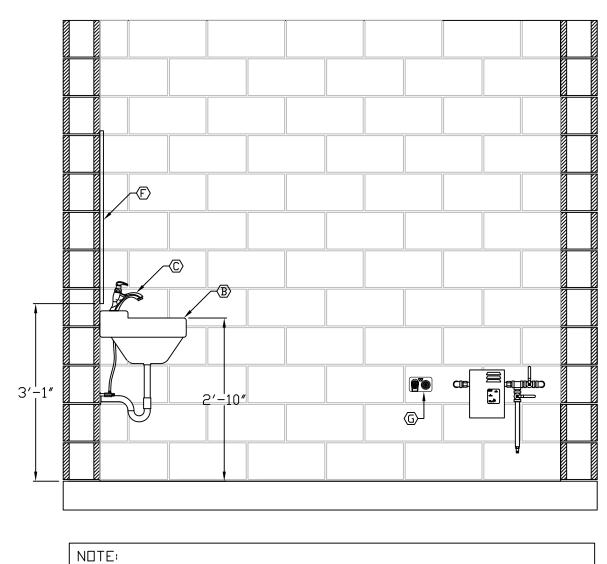






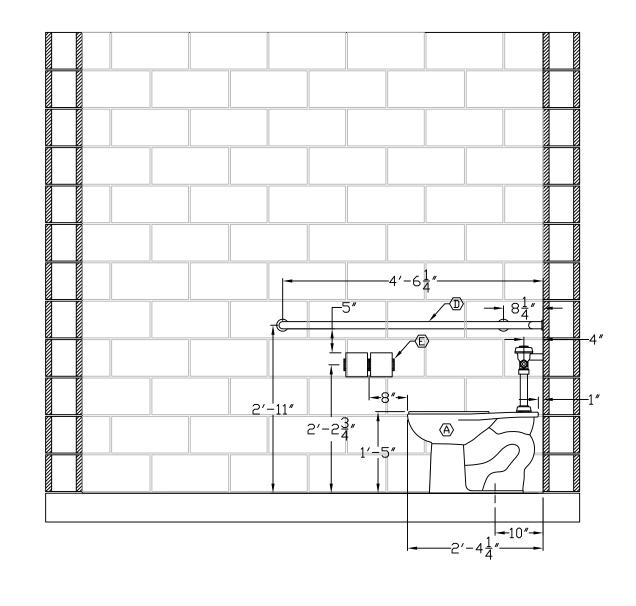


ELEVATION SECTION A-A



NOTE:
WATER SUPPLY AND DRAIN PIPES UNDER LAVATORY SHALL BE
INSULATED TO PROTECT AGAINST CONTACT, ALSO THERE SHALL
BE NO SHARP OR ABRASIVE SURFACES UNDER LAVATORY.

ELEVATION SECTION C-C

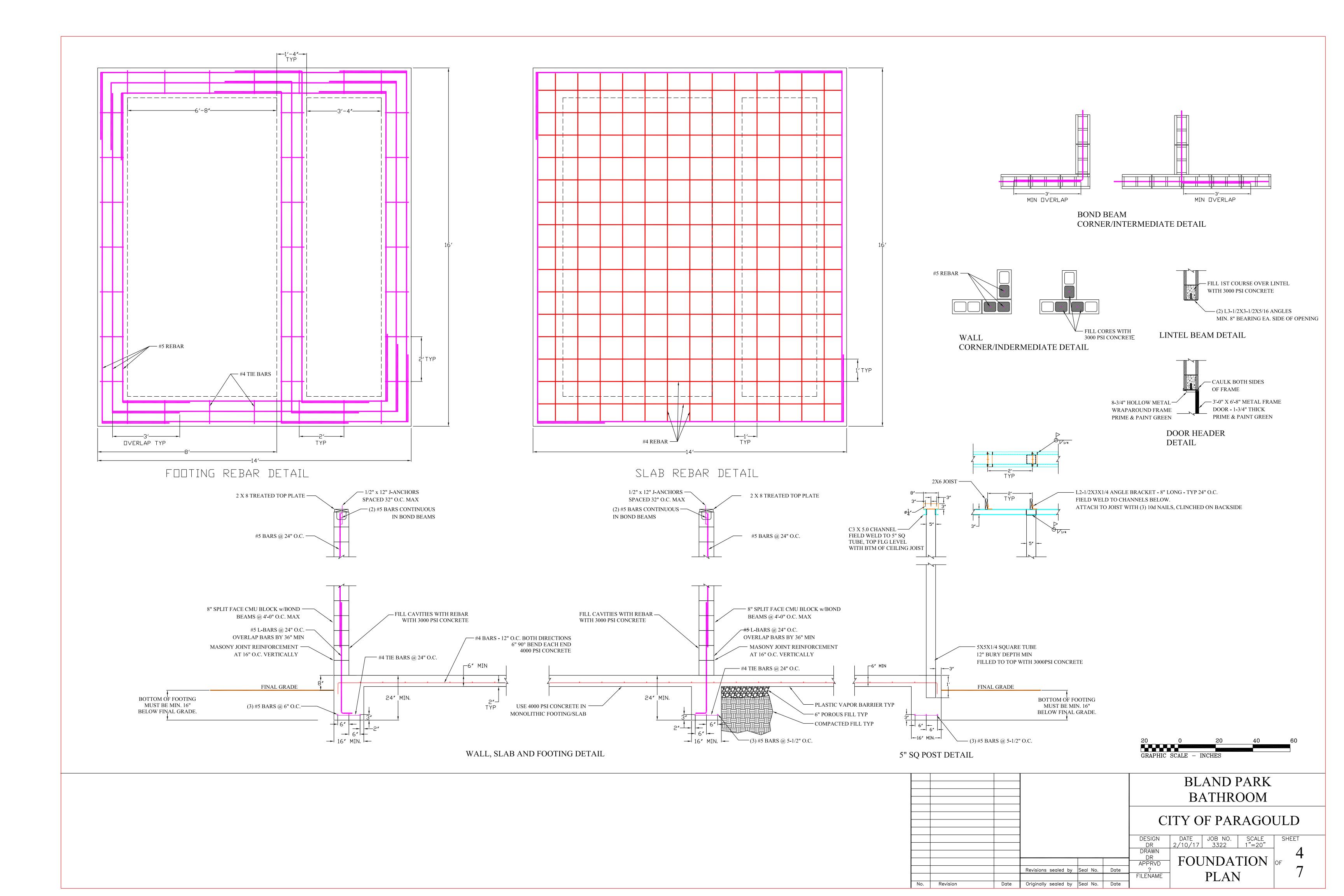


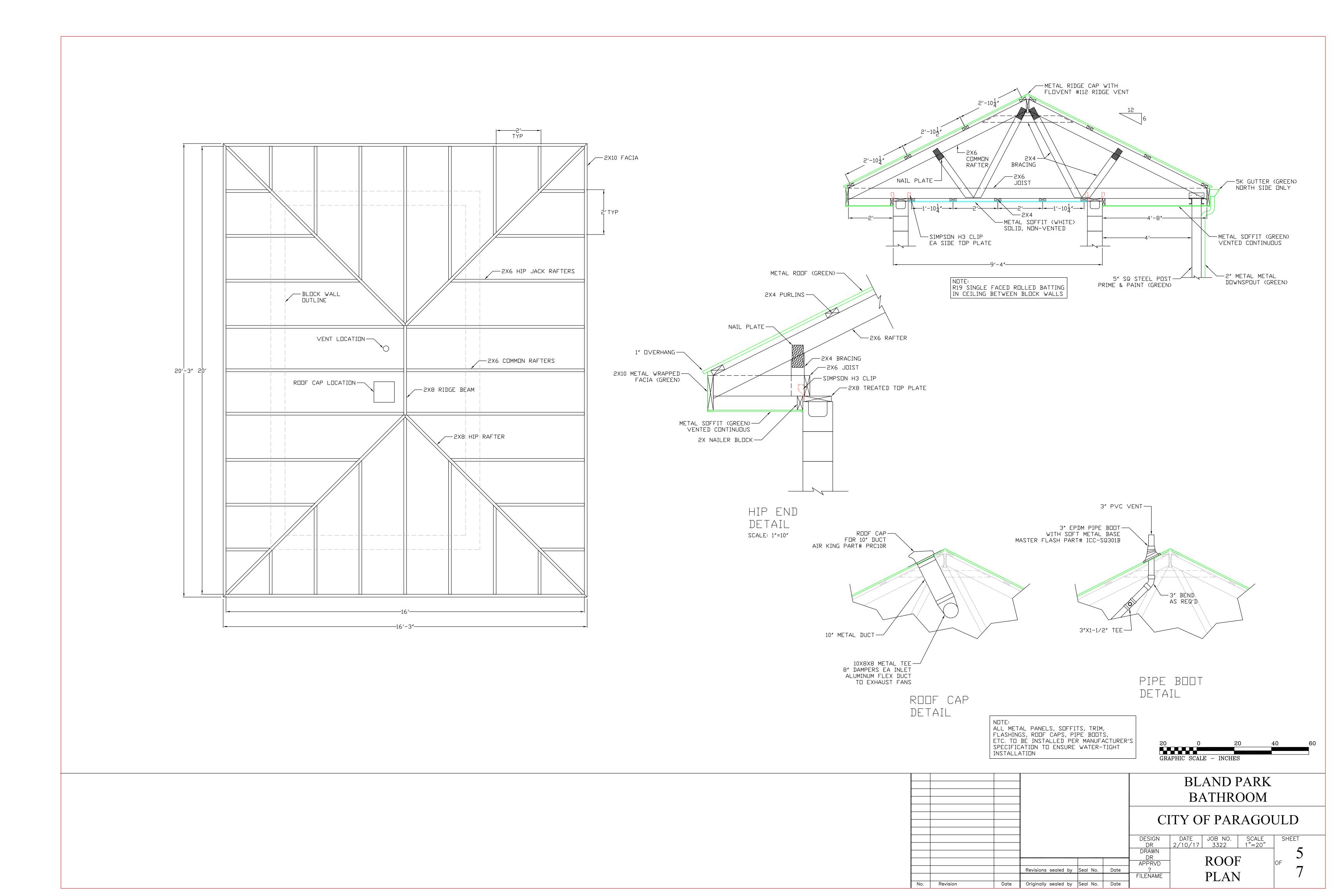
ELEVATION Section b-b

NOTE: CONTRACTOR TO COMPLY WITH CURRENT ADA STANDARDS

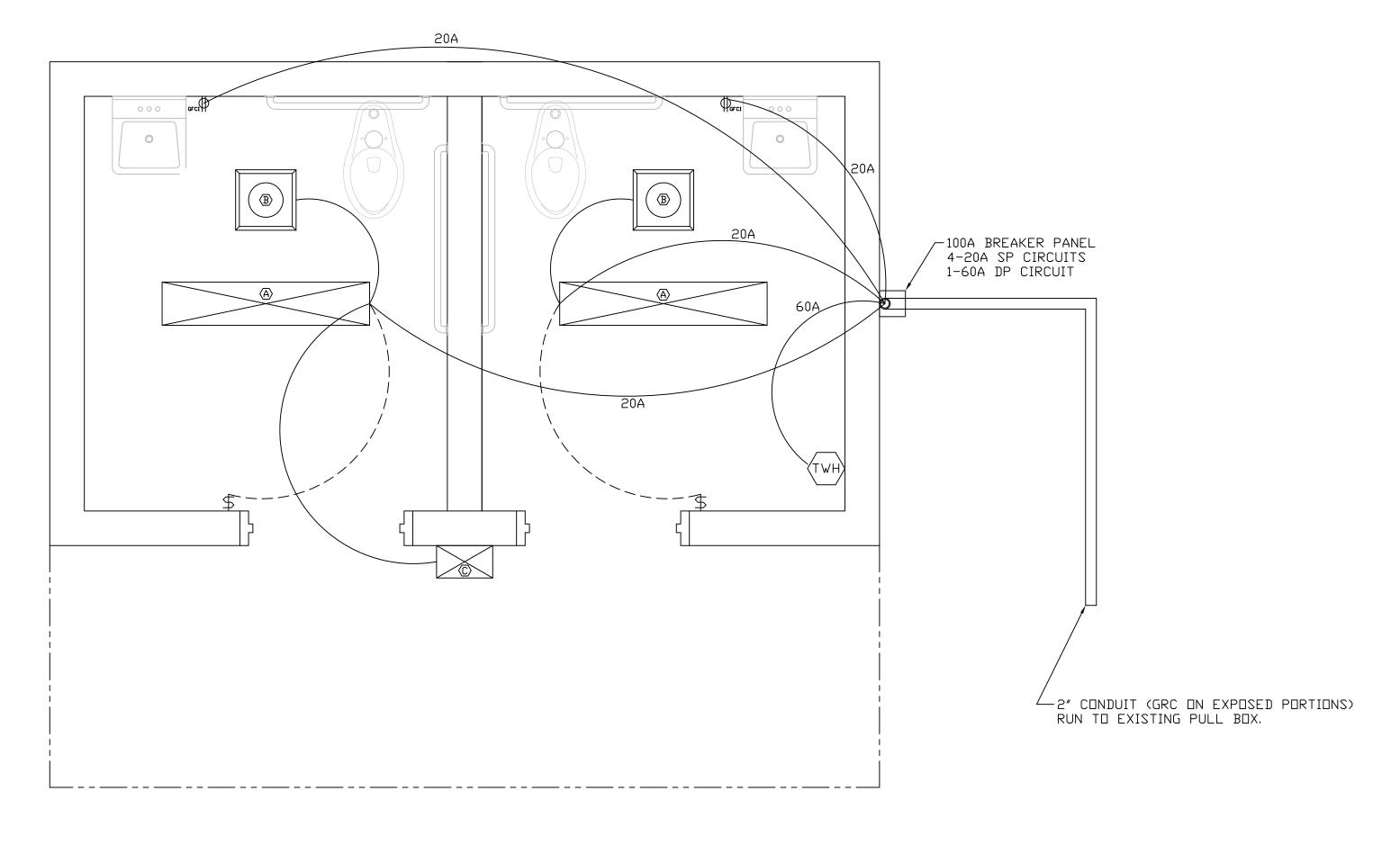
20	0	20	40	60
	\			
GRAPHI	C SCALE -	INCHES		

							BLAND PARK BATHROOM
						C]	ITY OF PARAGOULD
						DESIGN DR DRAWN DR	DATE JOB NO. SCALE SHEET 2/10/17 3322 1"=20"
No.	Revision	Date	Revisions sealed by  Originally sealed by	Seal No.	Date Date	APPRVD ? FILENAME	FLOOR PLAN 7



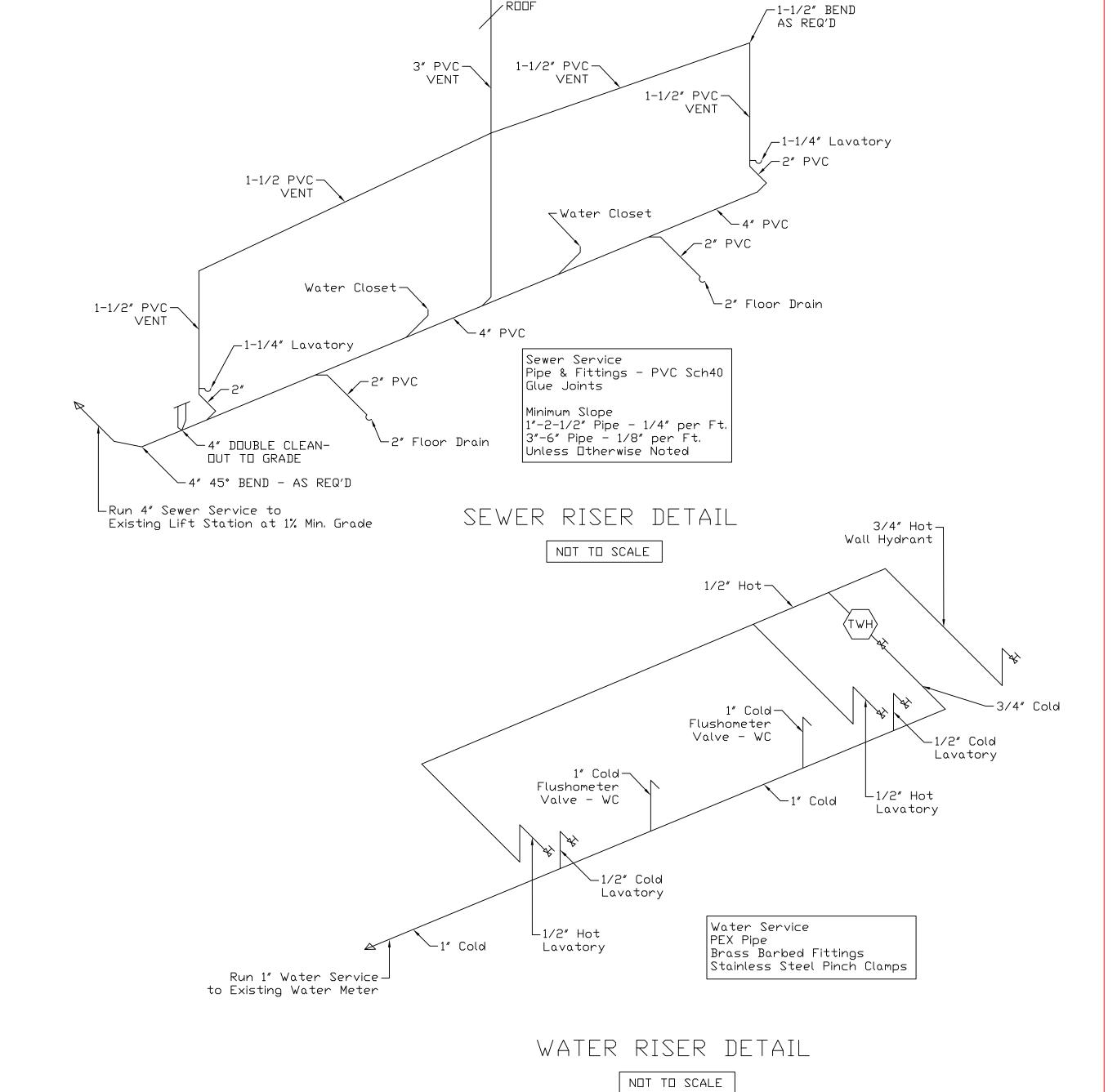


NOTE: ALL WIRING TO BE COPPER CONDUCTORS TYPE THHN, 12 GA MIN FOR 20A CIRCUITS, 6 AWG MIN FOR 60A CIRCUIT CONTRACTOR RESPONSIBLE FOR INSTALLING ALL FIXTURES, SWITCHES, RECEPTACLES, CONDUIT AND WIRING TO THE BREAKER PANEL, PLWC WILL SUPPLY AND INSTALL THE BREAKER PANEL AND THE CONDUIT FROM THE BREAKER PANEL TO THE PULL BOX, PLWC WILL PULL WIRE TO THE BREAKER PANEL AND MAKE-UP ALL CONNECTIONS AT THE BREAKER PANEL.



ELECTRICAL DETAIL

ITEM	QTY	DESCRIPTION	MANUFACTURER	MODEL	AMPS	COMMENTS
lack	N	4' LIGHT	LITHONIA	LBL44000LM80CRI40KN□DIMM∨□LT		41W LED 4000 LUMEN
B	N	EXHAUST FAN	BR□AN	L300	2.6 EA	
	N	LED WALL PACK	LITHONIA	TWR1LED50KMVOLTPE	0.34 EA	3400 LUMENS, PHOTO CONTROL
\$	ω	SPST SWITCH	HUBBELL	CSB120W	20 RATED	
Ф <sub>оегст</sub>	4	GFCI DUPLEX RECEPTACLE	HUBBELL	GFTWRST20W	20 RATED	
(√+)	1	ELECTRIC TANKLESS WATER HEATER	RHEEM	RTE13	54	240∨ - 13KW



/ROOF

CONTRACTOR TO COMPLY WITH CURRENT ARKANSAS STATE PLUMBING CODE AND NATIONAL ELECTRIC CODE

							BLAND PARK BATHROOM	
				- - -			CITY OF PARAGOUL	D
							DESIGN DATE JOB NO. SCALE SH 2/10/17 3322 1"=20"  DRAWN DR APPRVD ELECTRIC & OF	6 6
				,	Seal No.	Date	FILENAME RISER PLAN	7
1	No.	Revision	Date	Originally sealed by	Seal No.	Date		

```
APPLICABLE CODES AND STANDARDS:
  2006 INTERNATIONAL BUILDING CODE
  ACI 350 CODE REQUIREMENTS FOR ENVIRONMENTAL ENGINEERING CONCRETE STRUCTURES
  A.I.S.C. LOAD AND RESISTANCE FACTOR DESIGN SPECIFICATION FOR STRUCTURAL STEEL BUILDINGS (LRFD)
  A.W.S. D1.1, D1.3, D1.4
  BUILDING CODE REQUIREMENTS AND SPECIFICATIONS
  FOR MASONRY STRUCTURES- ACI530
  2012 ARKANSAS FIRE PREVENTION CODE
  2012 INTERNATIONAL BUILDING CODE
  2014 NATIONAL ELECTRIC CODE
  2004 ARKANSAS ENERGY CODE
                                          20 PSF REDUCIBLE
ROOF LIVE LOAD
ROOF SNOW LOAD (GROUND SNOW Pg=10 PSF) 16 PSF SNOW w/ RAIN SURCHARGE
BASIC WIND SPEED
                                          115 MPH
 EXPOSURE CLASSIFICATION
  IMPORTANCE FACTOR
SEISMIC LOADS
  SEISMIC DESIGN CATEGORY- D
  IMPORTANCE FACTOR 1.25
  SPECTRAL RESPONSE COEFFICIENT SDS = 1.0
  SPECTRAL RESPONSE COEFFICIENT SD1 = 0.60
  SITE CLASSIFICATION- D
MAXIMUM ALLOWABLE SOIL BEARING CAPACITY 2000 PSF
MATERIAL DATA:
CONCRETE & REINFORCING
  CONCRETE STRENGTH (fc @ 28 DAYS)
  WALLS, MAT SLABS
                                      4000 PSI
CEMENT TYPE
                           PORTLAND TYPE I/II
                           CONCRETE DESIGNATED AS 4500 PSI MUST HAVE A MAXIMUM
                           WATER-TO-CEMENT RATIO OF 0.42.
                           CONCRETE DESIGNATED AS 4500 PSI MUST HAVE A MAXIMUM
                           WATER SOLUBULE CHLORIDE ION CONTENT OF 0.10 BY WEIGHT
AGGREGATES
                           REGULAR WT. HARDROCK TYPE- ASTM C33
REINFORCING STEEL
                           ASTM A706, WELDABLE
                           NOTE: A615, GRADE 60 REBAR MAY BE USED BUT ONLY IF MILL
                           TESTING REPORTS SHOW ACTUAL YIELD DOES NOT EXCEED 78 KSI AND
                           ACTUAL TENSILE STRESS IS GREATER THAN 1.25 TIMES ACTUAL YIELD STRESS
WELDED WIRE FABRIC
                          ASTM A185
PREFORMED EXPN. JT. (1/2 IN) ASTM D1751
   STRUCTURAL STEEL (WIDE FLANGES)
                                                    ASTM A992, GRADE 50
    STRUCTURAL STEEL (ALL OTHER SHAPES)
                                                   ASTMA36
    STRUCTURAL SQUARE TUBING
                                                   ASTM A500, GRADE B (Fy = 46 \text{ KSI})
    STAINLESS STEEL
                                                    ASTMA304L
                                                   ASTM F1554, GRADE 36,
     ANCHOR RODS
     BOLTED CONNECTIONS
                                                    ASTMA325N
     STAINLESS STEEL BOLTED CONNECTIONS
                                                   ASTM A193 Gr. B8, CLASS 2
     WELDED CONNECTIONS
                                                   E70XX ELECTRODES
     WELDED CONNECTIONS (GALV. SURFACES)
                                                   E6010 OR E6011 ELECTRODES
     HEADED CONCRETE ANCHORS (HCA)
                                                   ASTM A108 GRADES C1010 through C1020 (Fu = 65 KSI)
MASONRY
     MASONRY STRENGTH (fm @ 28 DAYS)
                                          ASTM C90 - NORMAL WT.
    CONCRETE UNITS
                                           2800 PSI (BASED ON NET AREA)
     UNIT COMPRESSIVE STRENGTH
     MORTAR TYPE
                                           ASTM C270, TYPE S
                                           ASTM C476, MAX. AGGREGATE SIZE =3/8"
     GROUT TYPE
     MORTAR STRENGTH (28 DAY STRENGTH) 1800 PSI
     GROUT STRENGTH (28 DAY STRENGTH)
     WIRE REINFORCING
                                           ASTM A82, STD. LADDER TYPE - 9 GAGE
                                            ZINC COATED PER ASTEM A116 - (DUR-O-WALL
                                            OR APPROVED EQUAL AT 16" O.C. VERTICAL SPACING)
APPLICABLE CODES AND STANDARDS:
WOOD SECTIONS- SOUTHERN PINE, DOUGLAS FIR, HEM FIR, OR SPRUCE-PINE-FIR
MEMBER GRADES (UNLESS OTHERWISE INDICATED ON THE PLANS)
STUDS AND JOISTS
                                           NO. 2 OR BETTER
COLUMNS AND POSTS
HEADERS AND LEDGERS
                                          NO. 2 OR BETTER
PLYWOOD ROOF SHEATHING (OSB ACCEPTABLE): 5/8" APA SHEATHING, EXPOSURE I, SPAN RATING 32/16.
STRUCTURAL NOTES:
GENERAL:
ALL WORK SHALL CONFORM TO THE REQUIREMENTS OF THE APPLICABLE BUILDING CODE.
THE STRUCTURE HAS BEEN DESIGNED TO RESIST DESIGN LOADS ONLY AS A COMPLETED STRUCTURE.
APPLICATION OF ANY LOADS TO THE PARTIALLY COMPLETED STRUCTURE SHALL BE CONSIDERED BY THE
CONTRACTOR AND SO INCLUDED IN THE DESIGN OF SHORING, BRACING, FORMWORK, AND ANY OTHER SUPPORTING
ELEMENTS PROVIDED FOR CONSTRUCTION OF THE STRUCTURE.
WHERE CONSTRUCTION MATERIALS OR EQUIPMENT ARE TEMPORARILY STORED ON ROOF OR FLOOR FRAMING.
THEY SHALL BE DISTRIBUTED SO THAT THE DESIGN LIVE LOAD IS NOT EXCEEDED.
DO NOT BACKFILL AGAINST WALLS OR OTHER STRUCTURAL ELEMENTS UNTIL SUCH ELEMENTS HAVE REACHED
THEIR INTENDED STRENGTH, HAVE BEEN ADEQUATELY BRACED, AND/OR HAVE OTHER INTEGRAL STRUCTURAL
ELEMENTS IN PLACE WHICH ARE INTENDED TO RESIST THE LATERAL EARTH LOADS.
DETAILS ON THE DRAWINGS INDICATED AS "TYPICAL" APPLY IN ALL AREAS WHERE CONDITIONS SIMILAR TO THE
DETAIL OCCUR.
THE STRUCTURAL DRAWINGS ARE NOT INTENDED FOR USE AS SHOP ERECTION DRAWINGS. REPRODUCTION OF
THESE DRAWINGS IN LIEU OF PREPARATION OF SHOP ERECTION DRAWINGS SIGNIFIES THE USERS' ACCEPTANCE
THAT ALL INFORMATION SHOWN IS CORRECT AND APPROPRIATE FOR SHOP DRAWINGS AND THAT THE USER WILL BE
FULLY RESPONSIBLE FOR EXPENSES THAT MAY OCCUR FROM SAID ACCEPTANCE.
UNLESS SPECIFICALLY NOTED, THERE ARE NO PROVISIONS MADE FOR FUTURE FLOORS, ROOFS, OR OTHER LOADS.
COORDINATION / VERIFICATION:
CHECK AND VERIFY ALL DIMENSIONS AND EXISTING CONDITIONS AND REPORT ANY DISCREPANCIES TO THE
ENGINEER BEFORE PROCEEDING WITH ANY PHASE OF THE WORK.
ANY PROPRIETARY STRUCTURAL SYSTEMS THAT ARE COMPOSED OF COMPONENTS TO BE FIELD ERECTED SHALL
BE SUPERVISED BY THE SUPPLIER DURING MANUFACTURING, DELIVERY, HANDLING, STORAGE, AND ERECTION IN
ACCORDANCE WITH THE INSTRUCTIONS PREPARED BY THE SUPPLIER.
PROVIDE ADEQUATE STRUCTURAL FRAMING AS APPROVED BY THE ENGINEER FOR ALL REQUIRED MECHANICAL
OPENINGS THROUGH SLABS, WALLS, FLOOR DECK, ETC., AND SUPPORT OF ALL MECHANICAL EQUIPMENT. OPENINGS
SHALL NOT BE PERMITTED THROUGH BEAMS UNLESS SPECIFICALLY DETAILED BY THE ENGINEER.
CONCRETE REINFORCING:
CONCRETE BATCH DESIGN(S) SHALL BE PROPORTIONED AND PRODUCED IN ACCORDANCE WITH A.C.I. 318, IN
PARTICULAR CHAPTER 5, AND A.C.I. 301. MIX AND DELIVER IN ACCORDANCE WITH ASTM C94.
SLUMP REQUIREMENTS:
SLOPING SURFACES
                                                                           MAX. 3IN.
FOUNDATIONS
                                                                           MIN. 1 IN./ MAX. 4 IN. MAX.
CONCRETE W/ PLASTICIZERS
                                                                           MAX. 8 IN.
OTHER CONCRETE
                                                                           MIN. 1 IN./ MAX. 5 IN.
AIR ENTRAINMENT
                                                                           CONCRETE EXPOSED TO WEATHER- 5% MIN.
ADMIXTURES
                                                                           SUBMIT AS REQUIRED FOR APPROVAL
FLY ASH
                                                                           MAX. 20% OF CEMENT CONTENT
CONCRETE TEST CYLINDERS:
SAMPLING IN FIELD
                               ASTM C172 & C31
CYLINDER STRENGTH TESTS
                               ASTM C39
FREQUENCY OF STRENGTH TESTS ONE PER 100 CU. YDS. OR
    MIN. ONCE FOR EA. 5000 S.F. OF SLABS OR WALLS OR
    MIN. ONCE PER DAY FOR EACH TYPE OF MIX
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ONE STRENGTH TEST =AVG. STRENGTHS OF TWO CYLINDERS@ 28 DAYS.

CYLINDERS TO BE TESTED: 2@ 7 DAYS, 2@ 28 DAYS.

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CONCRETE TO CONCRETE COLD JOINTS- PROVIDE 1/4" INTENTIONALLY ROUGHENED SURFACE AT ALL HORIZONTAL
CONCRETE TO CMU INTERFACES -118" INTENTIONALLY ROUGHENED SURFACE AT ALL HORIZONTAL JOINTS
EXPOSED CORNERS: PROVIDE A3/4" CHAMFER AT ALL EXPOSED CONCRETE CORNERS WHERE INDICATED
CURING: CONCRETE SHALL BE MAINTAINED IN A MOIST CONDITION FOR A MINIMUM OF SEVEN DAYS AFTER ITS
PLACEMENT. IF FORMWORK IS REMOVED PRIOR TO SEVEN DAYS, APPLY MOIST CURING TO NEWLY EXPOSED
 SURFACES. APPROVED CURING COMPOUNDS MAY BE USED IN LIEU OF MOIST CURING.
 REINFORCING BAR WELDING: ABSOLUTELY NO WELDING OF REINFORCING BARS OR TORCHING TO BEND REINFORCING
BARS SHALL BE ALLOWED WITHOUT THE SPECIFIC APPROVAL OF THE ENGINEER.
 MINIMUM CONCRETE CLEAR COVER:
 PROVIDE THE FOLLOWING MINIMUM CONCRETE COVER OVER REINFORCING (FACE OF CONCRETE TO EDGE OF BAR)
UNLESS DETAILED OTHERWISE ON DRAWINGS:
CONCRETE CAST AGAINST AND
PERMANENTLY EXPOSED TO EARTH
CONCRETE WALLS AND MAT SLABS
SUSPENDED SLAB AT OBSERVATION LEVEL 1 1/2"
BAR SUPPORT ACCESSORIES SHALL BE PROVIDED IN ACCORDANCE WITH THE LATEST A.C.I. MANUAL OF STANDARD
PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES
 BEAM REINFORCING ON BAR BOLSTERS @ 4 FT. D.C. MAX.
SLAB REINFORCING ON BAR BOLSTERS @ 4 FT. O.C. MAX.
WITH SAND PLATES AS REQUIRED
                                    PLASTIC COATED OR STAINLESS STEEL LEGS
GONG EXPOSED TO VIEW
 REINFORCING SHOP DRAWINGS: REINFORCING SUPPLIER SHALL PROVIDE COMPLETE PLACEMENT AND FABRICATION
DRAWINGS FOR ALL REINFORCING INCLUDING THE LOCATION AND SIZE OF ALL ACCESSORIES AND SUPPORTS.
BASIS OF DESIGN: THE FOUNDATION SYSTEM DESIGN IS BASED ON THE MAXIMUM SOIL BARING CAPACITY OF 2000 PSI.
FOOTINGS SHALL BE PLACED ON NEAT, CLEAN AND DRY EXCAVATIONS. EXTREME CARE SHALL BE TAKEN WHEN
EXCAVATING NEAR THE BEARING SURFACE. FOOT TRAFFIC SHALL BE KEPT TO A MINIMUM NECESSARY TO PLACE
 THE CONTRACTOR SHALL PROVIDE FOR ADEQUATE DRAINAGE OF SURFACE WATER AWAY FROM THE STRUCTURE AND
EXCAVATED AREAS DURING CONSTRUCTION. THIS INCLUDES NECESSARY PUMPING, TRENCHING, BACKFILL AND/OR
DIKE CONSTRUCTION.
 PREFORMED PLASTIC WATERSTOP:
 PRODUCED FROM BLENDS OF REFINED HYDROCARBON RESINS AND PLASTICIZING COMPOUNDS REINFORCED WITH INERT
MINERAL FILLER, AND SHALL CONTAIN NO SOLVENTS, IRRITATING FUMES OR OBNOXIOUS ODORS, COMPLYING WITH
FEDERAL SPECIFICATION SS-S210A.
GRADE SUPPORTED SLABS:
REINFORCED CONCRETE SLAB ON GRADE:
SLAB THICKNESS: BUILDING FLOOR - 6" MIN
BUILDING SLAB PLACED THE REQ'D CLEAR DISTANCE FROM TOP OF SLAB
VAPOR BARRIER: PLASTIC
GRANULAR SUBBASE UNDER SLAB-ON-GRADE
MINIMUM THICKNESS: 6 IN.
GRADATION REQUIREMENTS:
100% PASSING THE 3/4" SIEVE
LESS THAN 15% PASSING THE 100 SIEVE
LESS THAN 2% PASSING THE 200 SIEVE
 SELECT FILL, WHERE REQUIRED TO ACHIEVE FINAL GRADE;
 CLEAN, INORGANIC, LOW-PLASTICITY SILT OR LEAN CLAY WITH THE FOLLOWING PROPERTIES:
 PLASTICITY INDEX (PI) RANGE
                                       5 TO 25
 MOISTURE CONTENT(% OF OPTIMUM)
                                      +2%, -3%
 MAXIMUM LOOSE LIFT
 COMPACTION
 FILL SHOULD EXTEND OUTWARD FROM THE EXTERIOR PERIMETER OF THE BUILDING AND FOOTINGS A DISTANCE
 EQUAL TO THE HEIGHT OF FILL OR 5'-0", WHICHEVER IS GREATER.
 EXISTING SUBGRADE:
 HEAVILY ROOT INFESTED TOPSOIL, PAVING, AND DEBRIS SHOULD BE STRIPPED AND DISCARDED. REMAINING
 EARTH SHALL BE SCARIFIED TO A DEPTH OF 12" AND RECOMPACTED TO AT LEAST 95% STANDARD PROCTOR
 COMPACTION (%OF MAXIMUM DRY DENSITY) SHALL BE DETERMINED USING ASTM D-698 STANDARD PROCTOR TEST
 CRACK CONTROL JOINTS (WHETHER CONSTRUCTION JOINTS OR SAWED JOINTS) IN SLABS ON GRADE SHALL OCCUR AS
 SHOWN AND ACROSS ALL DOOR OPENINGS. LOCATE JOINTS AT RE-ENTRANT CORNERS OF SLABS.
 MAXIMUM SPACING OF CONTROL JOINTS: 12'-0".
```

SEAL ALL EXPOSED CONSTRUCTION/CRACK CONTROL JOINTS AND EXPANSION JOINTS WITH POLYURETHANE TYPE SEALANT

**BEARING CONNECTIONS:** UNLESS OTHERWISE NOTED, ALL BEAM CONNECTIONS SHALL BE SIMPLE FRAMED SHEAR BEARING CONNECTIONS IN ACCORDANCE WITH THE AISC "SPECIFICATIONS FOR STRUCTURAL JOINTS USING A.S.T.M. A325 OR A490 BOLTS."

**DESIGN OF CONNECTIONS:** BEAM CONNECTIONS SHALL BE AS DETAILED ON THE PLANS. ALTERNATIVE CONNECTIONS, DESIGNED BY A LICENSED ENGINEER FOR THE FABRICATOR, MAY BE UTILIZED PROVIDED THE ALTERNATIVE CONNECTION PROVIDES THE SAME LOAD CARRYING CAPACITY OF THE ORIGINAL DESIGN.

STEEL PROTECTION: ALL STRUCTURAL STEEL SHALL BE CLEANED PER SSPC SP-2 HAND TOOL CLEANING OR SP-3 POWER TOOL CLEANING AND SHOP PAINTED WITH ONE COAT OF THE FABRICATOR'S STANDARD PRIMER (PAINT TYPE SSPC-PAINT 13 OR25). TOUCH UP SCARRED AREAS WITH THE SAME PAINT AFTER ERECTION.

SHOP DRAWINGS: STEEL FABRICATOR SHALL PROVIDE COMPLETE ERECTION AND FABRICATION DRAWINGS

**WELDING:** ALL WELDING SHALL BE PERFORMED BY QUALIFIED WELDERS PER AWS STANDARD QUALIFICATION PROCEDURES.

MASONRY/GROUT:

EACH SIDE OF WALL OPENINGS

STRUCTURAL STEEL:

**DESIGN CRITERIA:** MASONRY SHALL BE CONSTRUCTED PER THE REQUIREMENTS OF ACI 530.

UNIT TYPES: REFER TO ARCHITECTURAL DRAWINGS FOR CONCRETE MASONRY UNIT SIZE, FACE, COLOR, JOINTING, FTC

**MORTAR:** MORTAR SHALL BE TYPES AND BE PROPORTIONED PER THE BUILDING CODE REQUIREMENTS.

CMU REINFORCING: UNLESS NOTED OTHERWISE, REINFORCE ALL CMU WALLS WITH VERTICAL #5@16"0.C. FILL ALL REINFORCED CELLS WITH GROUT. MINIMUM GROUT BETWEEN MASONRY AND REINFORCING SHALL BE 1/2". MINIMUM GROUT BETWEEN PAIRS OF REINFORCING BARS SHALL BE 3/4".

PROVIDE #5 VERTICAL REINFORCING AT THE FOLLOWING LOCATIONS: AT ALL CORNERS OF CMU WALLS. AT ALL ENDS OF WALLS EACH SIDE OF CONTROL JOINTS

BOND BEAMS: (AT TOP OF ALL CMU WALLS AND LOCATED AT 4'-0" D.C. MAX. VERTICALLY) MINIMUM DEPTH UNLESS OTHERWISE NOTED= 8".

REINFORCING- (2) #5 CONTINUOUS. LAP SPLICE= 441NCHES

CORNER BARS- (2) #5 RIGHT ANGLE DOWELS THAT LAP A MINIMUM OF 3'-8" WITH ADJACENT BOND BEAM

REINFORCING. IF BOND BEAMS AT INTERSECTING WALLS MEET AT DIFFERENT ELEVATIONS, EXTEND BOTH BOND BEAMS AROUND INTERSECTING CORNER TO FIRST INTERIOR REINFORCED CELL, BUT NOT LESS THAN 4 FEET.

EXTEND BOND BEAMS THROUGH CONTROL JOINTS, BUT INTERRUPT BOND BEAM REINFORCING AT CONTROL JOINTS.

CMU LINTELS: PROVIDE (2) #5 BARS IN U-SHAPED LINTEL BEAM. GROUT U-SHAPED COURSE AND TWO ADDITIONAL COURSES IMMEDIATELY ABOVE THE LINTEL COURSE. PROVIDE A MINIMUM OF 81NCHES BEARING AT EACH END.

**CONTROL JOINTS:** EXCEPT WHERE OFFSETS OR SLIP JOINTS ARE SHOWN, CONTROL JOINTS SHALL BE A CONTINUOUS VERTICAL LINE FROM TOP OF FOOTING TO TOP OF MASONRY WALL. SPACE CONTROL JOINTS AS SHOWN ON THE PLANS.

**COLD WEATHER MASONRY CONSTRUCTION:** CONFORM TO "RECOMMENDED PRACTICES AND GUIDE SPECIFICATIONS FOR COLD WEATHER MASONRY CONSTRUCTION" BY INTERNATIONAL MASONRY INDUSTRY ALL-WEATHER COUNCIL.

BRICK MASONRY ATTACHMENT: BRICK TIES SHALL BE CONTINUOUS SINGLE WIRE OF SIZE W1.7 OR LARGER AT MAXIMUM SPACING OF 18" VERTICALLY. CORRUGATED SHEET METAL ANCHORS ARE NOT PERMITTED. PROVIDE AT LEAST ONE BRICK ANCHOR FOR EACH 2.0 SQUARE FEET OF WALL

PRECAST CONCRETE

**DESIGN CRITERIA:** MANUFACTURING PROCEDURES FOR ALL PRECAST, PRESTRESSED CONCRETE UNITS SHALL BE IN COMPLIANCE WITH PCI MNL-116, ACI301, AND ACI318 AND DESIGNED BY THE FABRICATOR. DESIGN CALCULATIONS SHALL BE FURNISHED WITH THE SHOP DRAWINGS BEARING THE SEAL OF A LICENSED ENGINEER IN THE STATE OF ARKANSAS
THE WEIGHT OF MECHANICAL EQUIPMENT AND THEIR OPENINGS SHALL BE INCLUDED IN THE DESIGN OF THE

APPROPRIATE UNITS AS REQUIRED. SHOW ALL LOADS USED IN THE DESIGN ON THE SHOP DRAWINGS.

**FABRICATOR QUALIFICATIONS:** FABRICATOR MUST BE A PRODUCER MEMBER OF THE PRESTRESSED CONCRETE INSTITUTE AND/OR PARTICIPATE IN ITS PLANT CERTIFICATION PROGRAM.

**CONNECTIONS:** EACH PRECAST UNIT SHALL HAVE CAST-IN-PLACE WELDING STRIPS FOR FASTENING THE UNITS TO SUPPORTING MEMBERS AND TO EACH OTHER: SIZE AND SPACING AS PER MANUFACTURER'S RECOMMENDATIONS.

**OPENINGS:** ALL OPENINGS IN THE PRECAST CONCRETE UNITS WHICH ARE LARGER THAN 6 INCHES IN DIAMETER SHALL BE PROVIDED BY THE PRECAST MANUFACTURER. LOCATE ALL OPENINGS ON THE SHOP DRAWINGS.

WOOD:

SHEATHING NAILING: COMMON WIRE NAILS SHALL BE USED AND PENETRATE SUPPORTING MEMBERS A MINIMUM 1 5/8". INDIVIDUAL PIECES OF SHEATHING SHALL NOT BE LESS THAN 24" IN THEIR SHORTEST PLAN DIRECTION NOR LESS THAN 8 SQ. FT. IN AREA: ROOF SHEATHING:

SHEET EDGES 10d AT 6" D.C.
INTERMEDIATE FRAMING MEMBERS 10d AT 12" D.C.
WALL SHEATHING:
SHEET EDGES 10d AT 6" O.C.
PANEL FIELD 10d AT 12" D.C.
PROVIDE BLOCKING AT ALL PANEL EDGES.

OPENINGS IN SHEATHING: EDGES OF ALL OPENINGS THROUGH SHEATHING SHALL BE NAILED PER THE REQUIRED EDGE NAILING ABOVE.

NOTCHING & CUTTING IN WOOD: JOISTS, RAFTERS AND BEAMS SHALL NOT BE NOTCHED, EXCEPT WHERE SHOWN IN DETAILS. OBTAIN ENGINEER'S APPROVAL FOR ANY HOLES THROUGH OR NOTCHES IN THE TOP OF HORIZONTAL MEMBERS.

NAILING SCHEDULE:- USE THE FOLLOWING TABLE UNLESS NOTED OTHERWISE ON THE PLANS OR DETAILS (COMMON WIRE NAILS OR EQUAL)
TABLE 2304.9.11N THE 20061NTERNATIONAL BUILDING CODE

WOOD CONNECTIONS: NOTATIONS ON DRAWINGS RELATING TO FRAMING CLIPS, JOISTS AND PURLIN HANGERS, AND OTHER CONNECTING DEVICES REFER TO CATALOG NUMBERS OF CONNECTORS MANUFACTURED BY THE SIMPSON STRONG-TIE COMPANY. EQUIVALENT DEVICES BY OTHER MANUFACTURERS MAY BE USED PROVIDED THEY HAVE ICBO APPROVAL FOR EQUAL LOAD CAPACITIES. WHERE SPECIFIC CONNECTORS ARE NOT INDICATED, SIMILAR DEVICES TO THOSE SHOWN SHALL BE USED. PROVIDE CONNECTING DEVICES WITH GALVANIZED COATING (G185- PERASTM A653) AND HOT-DIP FASTENERS (ASTM A153) WHEN USED WITH TREATED WOOD PRODUCTS.

**FRAMING AT WALL OPENINGS:** PROVIDE THREE- 2x8 HEADERS ON CRIPPLE STUDS, AND FULL HEIGHT DOUBLE STUDS EACH SIDE OF ALL OPENINGS IN STUD WALLS NOT DETAILED OTHERWISE.

**BLOCKING IN WALLS:** PROVIDE CONTINUOUS SOLID BLOCKING AT 6 FOOT MAXIMUM CENTERS IN ALL STUD WALLS OVER 8 FEET IN HEIGHT.

ANCHORAGE

**HEADED CONCRETE ANCHORS (HCA):** AUTOMATICALLY END WELDED IN THE SHOP OR FIELD IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATIONS. ANCHOR WELDS SHALL BE TESTED PER AWS SECTION 7.7.

POST-INSTALLED MECHANICAL ANCHORS: SEE DETAILS

INSTALL USING MINIMUM TORQUE, EMBEDMENTS, EDGE DISTANCES AND SPACING (UNLESS OTHERWISE NOTED) AS RECOMMENDED BY THE ANCHOR MANUFACTURER.

**POST-INSTALLED ADHESIVE ANCHORS:** SEE DETAILS INSTALLATION TO MEET MANUFACTURER'S RECOMMENDATIONS (UNLESS NOTED OTHERWISE)

INSTALLATION TO MEET MANUFACTURER'S RECOMMENDATIONS (UNLESS NOTED OTHERWISE) MIN. EMBEDMENTS, EDGE DISTANCES, SPACING, PROCEDURES, AND CURING TIME PRIOR TO LOADING. POST-INSTALLED ANCHORS SHALL BE LOCATED PER THE DETAILS.

AVOID CUTTING OR DAMAGING EXISTING REINFORCING. SHOULD LOCATIONS OF DRILLED HOLES BE FOUND DIRECTLY ALIGNED WITH REINFORCING BARS, NOTIFY ENGINEER FOR NECESSARY ADJUSTMENTS TO THE DESIGN.

POST-INSTALLED ANCHORS LOCATED IN PRESTRESSED MEMBERS SHALL NOT BE DRILLED UNTIL EXACT LOCATIONS OF TENDONS OR STRANDS ARE DETERMINED USING NON-DESTRUCTIVE METHODS.

